# **DRAINAGE STUDY**

For

**Allenstown Community Center** 

8 Whitten Street Allenstown, New Hampshire

March 18, 2015

Prepared For:

The Town of Allenstown, New Hampshire



Prepared By:



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## Allenstown Community Center 8 Whitten St. Allenstown, New Hampshire

#### PROJECT DESCRIPTION

The proposed project is located at 8 Whitten Street in the Town of Allenstown, New Hampshire. The project is situated on lot 112-267 which has an area of 1.741 acres and is owned by the Town of Allenstown. The parcel is known as the "Whitten Street Park" and is the current site of the Boys & Girls Club of Suncook.

The existing Boys & Girls Club site is comprised of two manufactured buildings, paved tennis and basketball courts, and parking accessed by a narrow driveway. The west part of the site slopes steeply down to Reynolds Street and is partially retained by a boulder wall. The aged existing facilities will be demolished to provide space for the new project.

The project includes construction of a new 9,400 square-foot two story building, parking both on the upper portion of the site and on Reynolds Street, a 20 ft wide access drive, sidewalks, a basketball court and associated landscaping, lighting and drainage improvements. The building will be sited on the existing steep slope facing Reynolds Street to provide ground level access to the first floor from Reynolds Street and to the second floor from the upper parking lot. The new Allenstown Community Center building will have a dual function of serving both the Boys & Girls Club and a senior center.

The drainage design was completed based on the USDA NRCS Web Soil Survey, which shows that the soils on the site and surrounding watersheds are Hydrological Soil Group (HSG) A soils. HSG A soils which have low runoff potential and high infiltration rates. The soil type associated with the site and surrounding neighborhood was identified as 598B, Windsor-Urban land complex, with 0 to 8 percent slopes.

The site poses some unique challenges such as a shallow groundwater table and is located within a groundwater management zone (GMZ), as defined by New Hampshire Department of Environmental Services Permit # GMP-198400003-A-003. Groundwater within the GMZ may contain dissolved gasoline-related compounds at concentrations greater than the ambient groundwater quality standards. The gasoline-related compounds are attributable to a release that occurred at the Former Bob's Citgo (circa 1972) which was located at the intersection of Main Street and Ferry Streets.

A closed municipal drainage system runs through the site from east to west starting on Main Street and terminating at an outfall into the river. A "Design Point" located on Reynolds Street was chosen for the watershed analysis. Design Point 1 includes runoff from watersheds collected by the upstream closed system, including stormwater from the existing Boys & Girls Club site which ties in to the Design Point, a 10 inch clay pipe running along Reynolds Street. Refer to the *Existing Conditions Narrative* and *Proposed Conditions Narrative* sections of this study for a detailed explanation of the Design Point under pre- and post-development conditions.

This study examines the hydraulic and hydrologic characteristics associated with the development of the proposed project. An analysis of the drainage impacts due to the construction of the project was completed. As a result, the proposed stormwater management system has been designed to best utilize the existing hydrologic and hydraulic patterns, minimize impacts to surrounding areas, and provide best management practices for water quality treatment measures.

Based on FEMA flood insurance rate map for Merrimack County community panel number 33013C0564E dated April 19, 2010, the parcel is not located within a 100-year or 500-year flood zone.

#### REPORT METHODOLOGY

The runoff analysis is based on analyzes of the 2, 10, 25, and 50-year 24-hour design storms using the SCS TR-55 method with Type-III, 24-hour storms. All runoff from this development is accounted for in the analysis presented.

The summaries and calculations for the peak pre- and post-development rates of runoff, supporting calculations and maps for the proposed project are included in this study.

On-site subcatchments were delineated from completed survey and GIS mapping. SCS Runoff Curve Numbers (CN) were developed for each area of generally similar properties within each Subcatchment. The nature of flow, flow length, type of surface (e.g. ground cover) and slope were characterized for each Subcatchment and reach (swale or pipe) to determine the time of concentration and maximum outflow rate. Soil types and Hydrologic Soil Groups were determined from the USDA NRCS Web Soil Survey data. A copy of the soil report is included in Appendix A.

#### DRAINAGE ANALYSIS METHODOLOGY

Technical Release 20 (TR-20) by the Natural Resources Conservation Service was utilized for modeling the surface water hydrology of the site. The model begins with a rainfall amount uniformly imposed on the watershed over a specified time distribution, 24 hours in this analysis. The mass rainfall is converted to mass runoff by using a Runoff CN. The determination of the CN is based on assessments of soil characteristics, vegetation type and condition, amount of impervious areas, interception and surface storage. The calculated runoff is then transformed into a hydrograph by using unit hydrograph theory and routing procedures that depend on runoff travel time through the individual segments of the watershed. Typically, various storage configurations and volumes are analyzed to adjust detention times and the hydrograph so that the downstream peak discharge is reduced to equal or less than pre-development conditions.

The analysis utilized the computerized TR-20 software as included in the HydroCAD package. Tabular hydrographs were generated for the following:

- 2-year event/24-hour duration
- 10-year event /24-hour duration
- 25-year event /24-hour duration
- 50-year event/24-hour duration



Time of Concentration ( $T_c$ ) for each sub-area was computed based on physical characteristics including surface type, Manning's Roughness Coefficient, flow length, 2-Year/24-Hour rainfall values, and gradients of the land.

#### TR-55 Analysis

The overall site pre- and post-development hydrographs were calculated utilizing the method detailed in Technical Release 55 (TR-55) " *Urban Hydrology for Small Watersheds*" as published by the United States Department of Agriculture Soil Conservation Service, "SCS", and revised in June of 1986. Tabular hydrographs were computed based on CN, T<sub>C</sub>, T<sub>t</sub>, area and precipitation input values. Rainfall precipitation data were also obtained from TR-55.

The SCS Method is based upon the SCS Runoff Equation:

Q= 
$$\frac{(P-la)^2}{(P-la) + S}$$

Where:

Q = Runoff in Inches
P = Rainfall in Inches
S = Potential Maximum Retention in Inches
Ia = Initial Abstraction in Inches

Note:

S = 1000/CN – 10 CN = Runoff Curve Number

Computations were executed using the "HydroCAD" release 9.10 for Windows computer software for storm sewer design and analysis from Applied Microcomputer System.

#### EXISTING CONDITIONS NARRATIVE

The 1.741-acre parcel is accessed by Whitten Street and has frontage along Reynolds Street along the western boundary. The northern boundary of the parcel generally abuts residential properties with frontage on Whitten Street. The southern boundary of the parcel generally abuts residential properties with frontage on Ferry Street. The eastern boundary abuts the Allenstown Fire Station located on Ferry Street as well as other residential properties. The existing site is currently located at 8 Whitten Street and contains two manufactured buildings each with a footprint of approximately 1,500 square feet. The site also has associated parking, tennis and basketball courts, as well as areas of grass, brush and tree cover.

Design Point 1 is the 10 inch clay drainage pipe on Reynolds Street, which outlets from the existing drain manhole adjacent to the site (DMH-2). DMH-2 appears to be an old sewer manhole of approximately 100 years old as evidenced by the brick invert and based upon discussions with Public Works personnel. This drainage study assumes that all upstream structures located on Reynolds Street tie in to this 10 inch clay main though some catch basin structures have obscured outlet pipes and are not specifically modeled in this analysis.



The 10 inch clay main receives existing flow from a closed municipal drainage system which serves a watershed area of approximately 10 acres. As the hydrologic model in this report predicts, this pipe is currently undersized for the 10 year, 24 hour storm but no known reports of backup in the system have been received by the Town.

The USDA NRCS Web Soil Survey determined that the existing on-site soils include Windsor-Urban land complex (Hydrologic Soil Group A). The existing ground cover includes impervious surfaces due to the existing building, parking lot, tennis and basketball courts, residential structures and pavement as well as some roadway drainage. A detailed and tabular summary of the flows associated with each existing Design Point can be found in the *Pre Development Analysis Criteria* section of this study.

A Pre-Development Watershed Map is included in Appendix D.

### PRE-DEVELOPMENT ANALYSIS CRITERIA

The complete HydroCAD analysis for pre-development conditions is included in Appendix B. Rainfall precipitation data was obtained TR-55. The analysis criteria used for the SCS TR-20 hydraulic analysis of the pre-development conditions are as follows:

- Storm Event Frequency: 2, 10, 25, and 50 year, 24-hr storms
- <u>Storm (yr, 24-hr)</u> <u>Rainfall Depth (inches)</u>

2	2.90
10	4.40
25	5.00
50	5.50

• Runoff Coefficients (CN)

>75% grass cover, good, HSG A	= 39
2 acre lots, 12% impervious, HSG A	= 46
1 acre lots, 20% impervious, HSG A	= 51
1/2 acre lots, 25% impervious, HSG A	= 54
¼ acre lots, 38% impervious, HSG A	= 61
Pavement, roofs, parking lots	= 98

#### SCS TR-20 Analysis

The pre development peak discharge from the project area for 2, 10, 25, and 50-year 24-hour design storms frequencies of 24-hour duration is:

Analysis Point	Inflow	2-year	10-year	25-year	50-year
	Area	Storm	Storm	Storm	Storm
	Acres	Flow (cfs)	Flow (cfs)	Flow (cfs)	Flow (cfs)
Design Point 1	9.3	0.8	5.0	7.7	10.1



#### PROPOSED CONDITIONS NARRATIVE

The proposed Allenstown Community Center project will include the demolition of all existing impervious surfaces including buildings and parking lot, tennis and basketball courts and access drive. New impervious surfaces to be constructed include the proposed building, parking, an access drive and basketball court. A Post-Development Watershed Map is included in Appendix D.

Under proposed conditions, Design Point 1 contains generally the same overall watershed as the pre-development conditions but has improved stormwater routing on the site and on Reynolds Street.

All stormwater runoff from the site is captured within the closed municipal drainage system.

#### POST-DEVELOPMENT ANALYSIS CRITERIA

The complete HydroCAD analysis for post-development conditions is included in Appendix C. The analysis criteria used for the SCS TR-20 hydraulic analysis of the post-development conditions are as follows:

- Storm Event Frequency: 2, 10, 25, and 50 years.
- SCS Rainfall Distribution: Type III

Storm	(yr, 24-hr)	Rainfall Depth (inches)
2		2.90
10		4.40
25		5.00
50		5.50

• Runoff Coefficients (CN):

= 39
= 46
= 51
= 54
= 61
= 98

#### SCS TR-20 Analysis

The post development peak discharges from the subject area for a 2-, 10-, 25-, 50-, and 100year storm frequency of 24-hour duration are:

Analysis Point	Inflow	2-year	10-year	25-year	50-year
	Area	Storm	Storm	Storm	Storm
	Acres	Flow (cfs)	Flow (cfs)	Flow (cfs)	Flow (cfs)
Design Point 1	9.5	1.5	6.1	9.9	12.4



#### ANALYSIS SUMMARY

#### Water Quantity

The pre and post-development peak discharges from the study area for the 2, 10, 25 and 50 year storm frequencies of 24-hour duration for Design Point 1 are compared below:

24 hr Design		Post	
Storm	Pre Development	Development	Difference
	Outflow (cfs)	Outflow (cfs)	Outflow (cfs)
2 yr	0.8	1.5	+0.7
10 yr	5.0	6.1	+1.1
25 yr	7.7	9.9	+2.2
50 yr	10.1	12.4	+2.3

As summarized in the above table, the post-development stormwater flows exceed predevelopment flows. Flow attenuation performed through detention or infiltration is not practical at this site due to the shallow groundwater table and the concentrations of gasoline related compounds in the groundwater.

#### Water Quality Treatment

Stormwater treatment for the proposed site will be provided by deep sumps catch basins and treatment swales as detailed on the design plans.

#### Temporary Erosion Control

Temporary erosion control measures and their locations are shown on the enclosed Grading, Drainage and Erosion Control Plan and Detail Drawings, and will be included in the construction plans for implementation. Placement of various erosion control devices including silt fences and swales will handle temporary erosion control. Existing drainage structures will be protected with inlet sediment bags. Grass swales will be stabilized with seeding and/or jute mats with check dams employed to detain sediment and reduce velocity.

#### CONCLUSION

Based on the unavoidable existing conditions of a site that is compromised due to gasoline concentrations in the groundwater, infiltration to the groundwater is not desirable. Groundwater beneath the Whitten Street Park is part of a Groundwater Management Zone described in New Hampshire Department of Environmental Protection Permit # GMP-1984-00003-A-003. Groundwater within this zone may contain dissolved volatile organic compounds at concentrations greater than the ambient water quality criteria. Numerous measures have been implemented to reduce the concentrations of VOCs in soil and groundwater. VOC concentrations in groundwater are decreasing and the size of the dissolved plume is shrinking as biodegradation and other natural attenuation processes occur. Recharge of stormwater on



the Whitten Street Park could influence groundwater resulting in undesirable migration of the plume. Stormwater recharge is also likely to be ineffective because groundwater at the site is close to ground surface, ranging from about 5 feet below ground surface on the eastern portion of the park to only a foot or two beneath the western portion of the park. Any recharge of stormwater would ultimately be captured by the French drain that must be designed to lower the groundwater elevation in the vicinity of the proposed building. There would be no net benefit of recharging storm water that would have to be removed by the French drain.

It is our recommendation that the minor increase in the 10 year storm is left undetained as it leaves the site, and it is our understanding that downstream drainage improvements to the direct receiving system will be made by the Town of Allenstown this year. All runoff from the site will be captured in the closed system and there will not be an increase in stormwater runoff to abutting properties. The new drainage system will replace old closed drainage piping and structures on the site that have been subject to groundwater infiltration. There should be no anticipated or significant downstream impacts expected to the drainage system after these improvements have been implemented.



# APPENDIX A





United States Department of Agriculture

Natural Resources

Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Merrimack and Belknap Counties, New Hampshire





Custom Soil Resource Report

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## **Map Unit Legend**

Merrimack and Belknap Counties, New Hampshire (NH609)				
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI	
598B	Windsor-Urban land complex, 0 to 8 percent slopes	14.3	100.0%	
Totals for Area of Interest		14.3	100.0%	

## **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas. An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Merrimack and Belknap Counties, New Hampshire

#### 598B—Windsor-Urban land complex, 0 to 8 percent slopes

#### **Map Unit Setting**

National map unit symbol: 9dm2 Elevation: 200 to 1,970 feet Mean annual precipitation: 40 to 50 inches Mean annual air temperature: 45 to 52 degrees F Frost-free period: 100 to 200 days Farmland classification: Not prime farmland

#### **Map Unit Composition**

Urban land: 40 percent Windsor and similar soils: 40 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Windsor**

#### Setting

Landform: Terraces Down-slope shape: Linear Across-slope shape: Linear

#### **Typical profile**

Oi - 0 to 1 inches: slightly decomposed plant material

H1 - 1 to 3 inches: loamy fine sand

- H2 3 to 26 inches: loamy fine sand
- H3 26 to 65 inches: fine sand

#### **Properties and qualities**

Slope: 3 to 15 percent Depth to restrictive feature: More than 80 inches Natural drainage class: Excessively drained Runoff class: Low Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Available water storage in profile: Low (about 5.2 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A

#### **Description of Urban Land**

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8s

#### **Minor Components**

#### Hinckley

Percent of map unit: 5 percent Landform: Terraces Down-slope shape: Linear Across-slope shape: Linear

#### Deerfield

Percent of map unit: 5 percent Landform: Terraces Down-slope shape: Linear Across-slope shape: Linear

#### Mashpee

Percent of map unit: 5 percent Landform: Depressions Down-slope shape: Concave Across-slope shape: Concave

# **Soil Information for All Uses**

## **Soil Properties and Qualities**

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

## **Soil Qualities and Features**

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

## Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

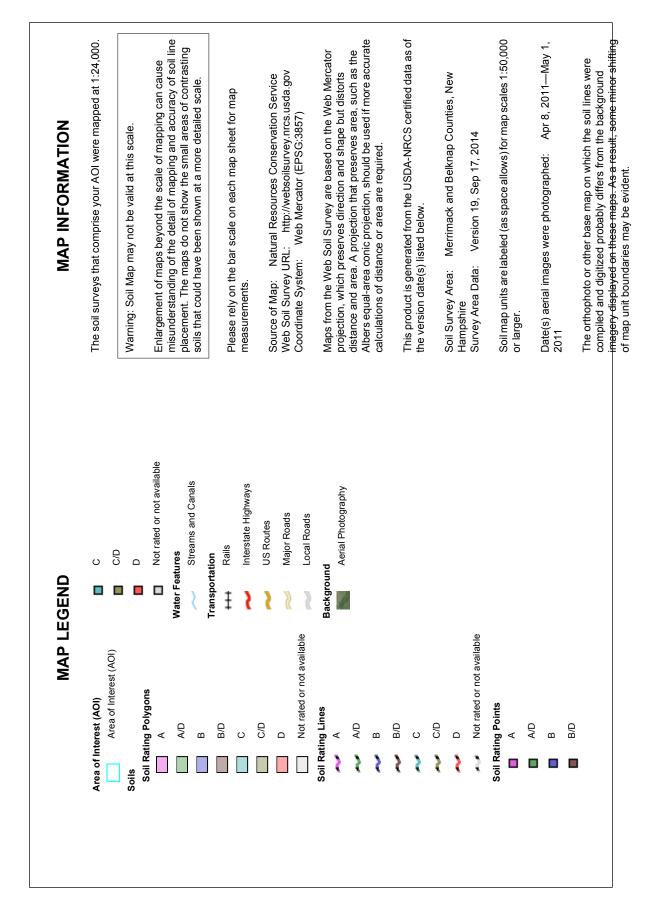
Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



Custom Soil Resource Report



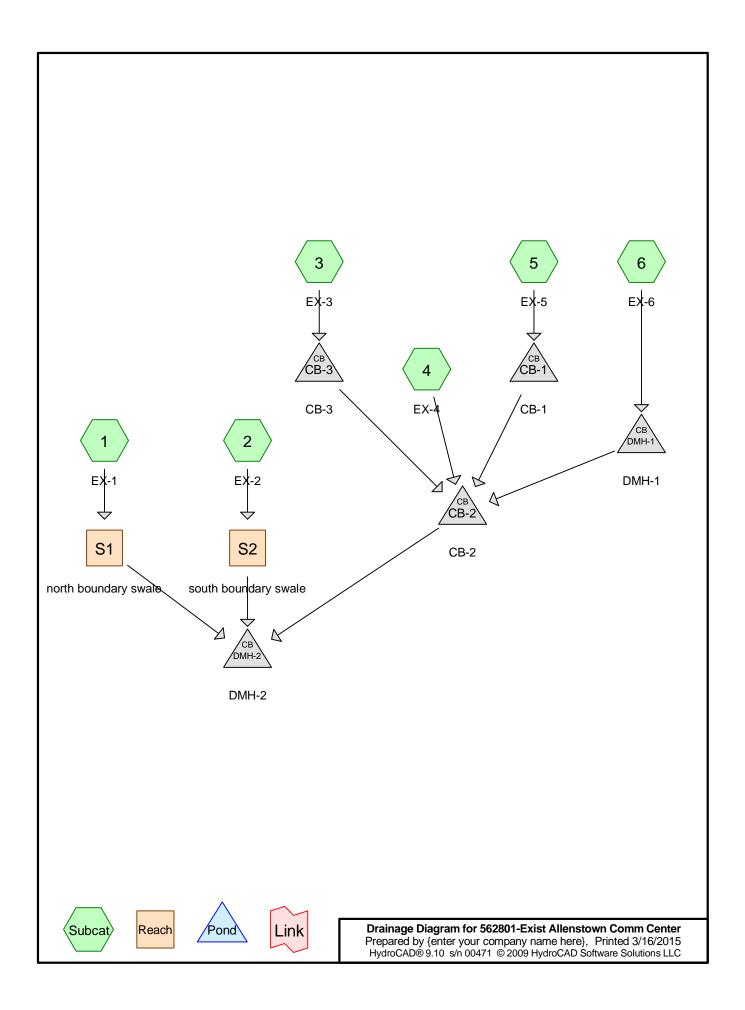
## Table—Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Merrimack and Belknap Counties, New Hampshire (NH609)					
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI	
598B	Windsor-Urban land complex, 0 to 8 percent slopes	A	14.3	100.0%	
Totals for Area of Intere	st		14.3	100.0%	

## Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

APPENDIX B



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## Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
1.096	39	>75% Grass cover, Good, HSG A (1, 2, 3, 4, 5)
0.423	46	2 acre lots, 12% imp, HSG A (3)
1.096	51	1 acre lots, 20% imp, HSG A (1)
4.234	54	1/2 acre lots, 25% imp, HSG A (4, 5, 6)
1.614	61	1/4 acre lots, 38% imp, HSG A (1, 2)
0.475	98	Paved parking, HSG A (1, 2, 3)
0.144	98	Reynolds, Whitten Pavement (1)
0.069	98	Unconnected roofs, HSG A (2)
0.189	98	reynolds (2)
9.341		TOTAL AREA

## Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
9.007	HSG A	1, 2, 3, 4, 5, 6
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.334	Other	1, 2
9.341		TOTAL AREA

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Liı	ne#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Fill (inches)
	1	6	0.00	0.00	286.0	0.0400	0.013	8.0	0.0	0.0
	2	CB-1	274.27	273.36	18.0	0.0506	0.010	8.0	0.0	0.0
	3	CB-2	271.38	254.10	266.0	0.0650	0.025	8.0	0.0	0.0
	4	CB-3	272.42	271.38	60.0	0.0173	0.025	8.0	0.0	0.0
	5	DMH-1	275.43	271.48	136.0	0.0290	0.013	8.0	0.0	0.0
	6	DMH-2	253.24	246.24	200.0	0.0350	0.013	10.0	0.0	0.0

## Pipe Listing (all nodes)

Type III 24-hr 2-yr Rainfall=2.90" Printed 3/16/2015 Page 5

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> Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: EX-1	Runoff Area=106,619 sf 31.99% Impervious Runoff Depth=0.24" Flow Length=887' Tc=17.1 min CN=58 Runoff=0.23 cfs 0.049 af
Subcatchment 2: EX-2	Runoff Area=82,355 sf 43.90% Impervious Runoff Depth=0.46"
Flow Ler	agth=580' Slope=0.0460 '/' Tc=9.9 min CN=65 Runoff=0.61 cfs 0.073 af
Subcatchment 3: EX-3	Runoff Area=26,342 sf 24.56% Impervious Runoff Depth=0.13" Flow Length=551' Tc=10.7 min CN=53 Runoff=0.02 cfs 0.006 af
Subcatchment 4: EX-4	Runoff Area=680 sf 11.65% Impervious Runoff Depth=0.02"
Flow Ler	agth=302' Slope=0.0500 '/' Tc=9.3 min CN=46 Runoff=0.00 cfs 0.000 af
Subcatchment 5: EX-5	Runoff Area=50,148 sf 21.62% Impervious Runoff Depth=0.11"
Flow Leng	h=561' Slope=0.0600 '/' Tc=11.1 min CN=52 Runoff=0.02 cfs 0.010 af
Subcatchment 6: EX-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=0.15" Flow Length=698' Tc=13.0 min CN=54 Runoff=0.12 cfs 0.040 af
Reach S1: north boundary swale	Avg. Flow Depth=0.03' Max Vel=1.03 fps Inflow=0.23 cfs 0.049 af
n=0.03	5 L=280.0' S=0.0571 '/' Capacity=117.19 cfs Outflow=0.22 cfs 0.049 af
Reach S2: south boundary swale n=0.03	Avg. Flow Depth=0.04' Max Vel=1.33 fps Inflow=0.61 cfs 0.073 af 5 L=285.0' S=0.0702 '/' Capacity=403.00 cfs Outflow=0.58 cfs 0.073 af
Pond CB-1: CB-1	Peak Elev=274.34' Inflow=0.02 cfs 0.010 af
8.0	" Round Culvert n=0.010 L=18.0' S=0.0506 '/' Outflow=0.02 cfs 0.010 af
Pond CB-2: CB-2	Peak Elev=271.59' Inflow=0.15 cfs 0.056 af
8.0"	Round Culvert n=0.025 L=266.0' S=0.0650 '/' Outflow=0.15 cfs 0.056 af
Pond CB-3: CB-3	Peak Elev=272.51' Inflow=0.02 cfs 0.006 af
8.0	" Round Culvert n=0.025 L=60.0' S=0.0173 '/' Outflow=0.02 cfs 0.006 af
Pond DMH-1: DMH-1	Peak Elev=275.62' Inflow=0.12 cfs 0.040 af
8.0"	Round Culvert n=0.013 L=136.0' S=0.0290 '/' Outflow=0.12 cfs 0.040 af
Pond DMH-2: DMH-2	Peak Elev=253.72' Inflow=0.77 cfs 0.179 af
10.0"	Round Culvert n=0.013 L=200.0' S=0.0350 '/' Outflow=0.77 cfs 0.179 af
Total Runoff Area =	9.341 ac Runoff Volume = 0.179 af Average Runoff Depth = $0.23^{\circ}$

69.81% Pervious = 6.521 ac 30.19% Impervious = 2.820 ac

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#### Summary for Subcatchment 1: EX-1

Runoff = 0.23 cfs @ 12.49 hrs, Volume= 0.049 af, Depth= 0.24"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

A	rea (sf)	CN E	Description		
	47,758	51 1	acre lots,	20% imp, H	ISG A
	28,151	61 1	/4 acre lots	s, 38% imp	, HSG A
	16,851	39 >	75% Gras	s cover, Go	ood, HSG A
	7,572	98 F	Paved park	ing, HSG A	
*	6,287	98 F	Reynolds, V	Vhitten Pav	rement
	106,619	58 V	Veighted A	verage	
	72,511	6	8.01% Per	vious Area	
	34,108	3	1.99% Imp	pervious Ar	ea
Тс	Length	Slope			Description
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
		•			Description Sheet Flow,
(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, Grass: Short n= 0.150 P2= 2.90"
(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, Grass: Short n= 0.150 P2= 2.90" Shallow Concentrated Flow,
<u>(min)</u> 6.6	(feet) 100	(ft/ft) 0.0600 0.0600	(ft/sec) 0.25		Sheet Flow, Grass: Short n= 0.150 P2= 2.90" Shallow Concentrated Flow, Woodland Kv= 5.0 fps
<u>(min)</u> 6.6	(feet) 100	(ft/ft) 0.0600	(ft/sec) 0.25		Sheet Flow, Grass: Short n= 0.150 P2= 2.90" Shallow Concentrated Flow, Woodland Kv= 5.0 fps Shallow Concentrated Flow,
<u>(min)</u> 6.6 10.4	(feet) 100 763	(ft/ft) 0.0600 0.0600	(ft/sec) 0.25 1.22		Sheet Flow, Grass: Short n= 0.150 P2= 2.90" Shallow Concentrated Flow, Woodland Kv= 5.0 fps

### Summary for Subcatchment 2: EX-2

	Runoff	=	0.61 cfs @	12.18 hrs, V	/olume=	0.073 af, Dep	oth= 0.46"
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Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

	Area (sf)	CN	Description
	42,149	61	1/4 acre lots, 38% imp, HSG A
	18,083	39	>75% Grass cover, Good, HSG A
*	8,243	98	reynolds
	3,019	98	Unconnected roofs, HSG A
	8,876	98	Paved parking, HSG A
	1,985	39	>75% Grass cover, Good, HSG A
	82,355	65	Weighted Average
	46,200		56.10% Pervious Area
	36,155		43.90% Impervious Area
	3,019		8.35% Unconnected

Prepare	d by {en	<b>llenstown Co</b> ter your compa s/n 00471 © 2009	ny name here	Type III 24-hr 2-yr Rainfall=2.90" e} Printed 3/16/2015 ftware Solutions LLC Page 7
Tc (min)	Length (feet)	Slope Veloo (ft/ft) (ft/s		Description
7.4	100		23	Sheet Flow,
2.5	480	0.0460 3	22	Grass: Short n= 0.150 P2= 2.90" Shallow Concentrated Flow, Grassed Waterway Kv= 15.0 fps
9.9	580	Total		· · ·
			Summary fo	r Subcatchment 3: EX-3
Runoff	=	0.02 cfs @ 1	2.53 hrs, Volu	ume= 0.006 af, Depth= 0.13"
		R-20 method, U <sup>·</sup> Rainfall=2.90''	H=SCS, Time	Span= 0.00-48.00 hrs, dt= 0.05 hrs
A	rea (sf)	CN Descrip	tion	
	18,409		ots, 12% imp, I	
	3,673		rass cover, G	
	4,260		arking, HSG A	4
	26,342		ed Average Pervious Area	
	19,873 6,469		Impervious Area	
	0,409	24.0070		ca
Тс	Length	Slope Velo	ty Capacity	Description
(min)	(feet)	(ft/ft) (ft/s		·
6.6	100	0.0600 0	25	Sheet Flow,
				Grass: Short n= 0.150 P2= 2.90"
3.8	387	0.0600 1	71	Shallow Concentrated Flow,
0.3	64	0.0300 3	52	Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,
0.5	64	0.0300 3	52	Paved Kv= 20.3 fps
10.7	551	Total		
			Summary fo	r Subcatchment 4: EX-4
Runoff	=	0.00 cfs @ 1	6.91 hrs, Volu	ume= 0.000 af, Depth= 0.02"
		R-20 method, U <sup>·</sup> Rainfall=2.90''	H=SCS, Time	Span= 0.00-48.00 hrs, dt= 0.05 hrs
А	rea (sf)	CN Descrip	tion	
	317		lots, 25% imp	o, HSG A
	363		rass cover, G	
	<u> </u>	40 \\/aimbt		

36339>75% Grass cover, Good68046Weighted Average60188.35% Pervious Area7911.65% Impervious Area

Prepare	d by {ent	ter your o	company	n Center name here droCAD Sof	Type III 24-hr 2-yr Rainfall=2.90"e}Printed 3/16/2015tware Solutions LLCPage 8
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.1	100	0.0500	0.23		Sheet Flow,
2.2	202	0.0500	1.57		Grass: Short n= 0.150 P2= 2.90" Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
9.3	302	Total			
			Sur	nmary foi	r Subcatchment 5: EX-5
Runoff	=	0.02 cfs	s@ 12.92	2 hrs, Volu	me= 0.010 af, Depth= 0.11"
		R-20 meth Rainfall		SCS, Time S	Span= 0.00-48.00 hrs, dt= 0.05 hrs
A	rea (sf)		escription		
	43,372 6,776			s, 25% imp, s cover, Gc	, HSG A bod, HSG A
	50,148 39,305 10,843	52 W 78	/eighted A 8.38% Per		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.6	100	0.0600	0.25		Sheet Flow,
4.5	461	0.0600	1.71		Grass: Short n= 0.150 P2= 2.90" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
11.1	561	Total			
			Sur	nmary foi	r Subcatchment 6: EX-6
Runoff	=	0.12 cfs	s@ 12.53	3 hrs, Volu	me= 0.040 af, Depth= 0.15"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

Area (sf)	CN	Description
140,742	54	1/2 acre lots, 25% imp, HSG A
105,557		75.00% Pervious Area
35,186		25.00% Impervious Area

Type III 24-hr 2-yr Rainfall=2.90"
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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	100	0.0380	0.21		Sheet Flow, Grass: Short n= 0.150 P2= 2.90"
4.3	312	0.0300	1.21		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps
0.7	286	0.0400	6.92	2.42	<b>Pipe Channel, Offsite Piped Flow from East</b> 8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17' n= 0.013 Clay tile

13.0 698 Total

#### Summary for Reach S1: north boundary swale

Inflow Area =	2.448 ac, 31.99% Impervious, Inflow I	Depth = 0.24" for 2-yr event
Inflow =	0.23 cfs @ 12.49 hrs, Volume=	0.049 af
Outflow =	0.22 cfs @ 12.54 hrs, Volume=	0.049 af, Atten= 2%, Lag= 3.1 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 1.03 fps, Min. Travel Time= 4.5 min Avg. Velocity = 0.56 fps, Avg. Travel Time= 8.4 min

Peak Storage= 60 cf @ 12.54 hrs Average Depth at Peak Storage= 0.03' Defined Flood Depth= 1.00', Capacity at Flood Depth= 117.19 cfs Bank-Full Depth= 1.00', Capacity at Bank-Full= 117.19 cfs

 $6.00' \times 1.00'$  deep channel, n= 0.035 Earth, dense weeds Side Slope Z-value= 10.0 '/' Top Width= 26.00' Length= 280.0' Slope= 0.0571 '/' Inlet Invert= 276.00', Outlet Invert= 260.00'



Summary for Reach S2: south boundary swale

Inflow Area	=	1.891 ac, 4	I3.90% Imp	ervious,	Inflow	Depth =	0.46"	for 2-y	/r event
Inflow =	=	0.61 cfs @	12.18 hrs,	Volume	=	0.073	af	-	
Outflow =	=	0.58 cfs @	12.24 hrs,	Volume	=	0.073	af, At	ten= 5%,	Lag= 3.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 1.33 fps, Min. Travel Time= 3.6 min Avg. Velocity = 0.71 fps, Avg. Travel Time= 6.7 min

Peak Storage= 124 cf @ 12.24 hrs Average Depth at Peak Storage= 0.04' Defined Flood Depth= 1.50', Capacity at Flood Depth= 403.00 cfs

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Bank-Full Depth= 1.50', Capacity at Bank-Full= 403.00 cfs

10.00' x 1.50' deep channel, n= 0.035 Earth, dense weeds Side Slope Z-value= 10.0 '/' Top Width= 40.00' Length= 285.0' Slope= 0.0702 '/' Inlet Invert= 280.00', Outlet Invert= 260.00'

Summary for Pond CB-1: CB-1

Inflow Area =	1.151 ac, 21.62% Impervious, Inflow D	epth = 0.11" for 2-yr event
Inflow =	0.02 cfs @ 12.92 hrs, Volume=	0.010 af
Outflow =	0.02 cfs @ 12.92 hrs, Volume=	0.010 af, Atten= 0%, Lag= 0.0 min
Primary =	0.02 cfs @ 12.92 hrs, Volume=	0.010 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 274.34' @ 12.92 hrs Flood Elev= 275.57'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 274.27'
 8.0" Round Culvert
 L= 18.0'
 RCP, sq.cut end projecting, Ke= 0.500

 Inlet / Outlet Invert=
 274.27'
 273.36'
 S= 0.0506 '/'
 Cc= 0.900

 n=
 0.010
 PVC, smooth interior
 PVC
 PVC

Primary OutFlow Max=0.02 cfs @ 12.92 hrs HW=274.34' TW=271.56' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.02 cfs @ 0.90 fps)

#### Summary for Pond CB-2: CB-2

Inflow Area	=	5.003 ac, 2	24.13% Impervious	, Inflow Depth = (	0.14" for 2-yr event
Inflow =	=	0.15 cfs @	12.54 hrs, Volum	e= 0.056 a	f
Outflow =	=	0.15 cfs @	12.54 hrs, Volum	e= 0.056 a	f, Atten= 0%, Lag= 0.0 min
Primary =	=	0.15 cfs @	12.54 hrs, Volum	e= 0.056 a	f

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 271.59' @ 12.54 hrs Flood Elev= 275.28'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.38'	<b>8.0" Round Culvert</b> L= 266.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 271.38' / 254.10' S= 0.0650 '/' Cc= 0.900 n= 0.025 Corrugated metal

Primary OutFlow Max=0.15 cfs @ 12.54 hrs HW=271.59' TW=253.70' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.15 cfs @ 1.56 fps) Prepared by {enter your company name here} HydroCAD® 9.10 s/n 00471 © 2009 HydroCAD Software Solutions LLC

## Summary for Pond CB-3: CB-3

			56% Impervious, Inflow Depth = $0.13$ " for 2-yr event		
Inflow	=	0.02 cts @ 12	2.53 hrs, Volume= 0.006 af		
Outflow	=	0.02 cfs @ 12	2.53 hrs, Volume= 0.006 af, Atten= 0%, Lag= 0.0 min		
Primary	=	0.02 cfs @ 12	2.53 hrs, Volume= 0.006 af		
Peak Ele	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 272.51' @ 12.53 hrs Flood Elev= 274.22'				
Device	Routing	Invert	Outlet Devices		
#1	Primary	272.42'	8.0" Round Culvert L= $60.0'$ RCP, sq.cut end projecting, Ke= $0.500$ Inlet / Outlet Invert= $272.42' / 271.38'$ S= $0.0173' / Cc= 0.900$ n= $0.025$ Corrugated metal		

Primary OutFlow Max=0.02 cfs @ 12.53 hrs HW=272.51' TW=271.59' (Dynamic Tailwater)

#### Summary for Pond DMH-1: DMH-1

Inflow Area =	3.231 ac, 25.00% Impervious, Inflow	Depth = 0.15" for 2-yr event
Inflow =	0.12 cfs @ 12.53 hrs, Volume=	0.040 af
Outflow =	0.12 cfs @ 12.53 hrs, Volume=	0.040 af, Atten= 0%, Lag= 0.0 min
Primary =	0.12 cfs @ 12.53 hrs, Volume=	0.040 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 275.62' @ 12.53 hrs Flood Elev= 281.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	275.43'	<b>8.0" Round Culvert</b> L= 136.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 275.43' / 271.48' S= 0.0290 '/' Cc= 0.900 n= 0.013 Clay tile

Primary OutFlow Max=0.12 cfs @ 12.53 hrs HW=275.61' TW=271.59' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.12 cfs @ 1.46 fps)

#### Summary for Pond DMH-2: DMH-2

Inflow Area =	9.341 ac, 30.19% Impervious, Inflow	Depth = 0.23" for 2-yr event
Inflow =	0.77 cfs @ 12.45 hrs, Volume=	0.179 af
Outflow =	0.77 cfs @ 12.45 hrs, Volume=	0.179 af, Atten= 0%, Lag= 0.0 min
Primary =	0.77 cfs @ 12.45 hrs, Volume=	0.179 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 253.72' @ 12.45 hrs Flood Elev= 258.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	253.24'	10.0" Round Culvert

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L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.24' / 246.24' S= 0.0350 '/' Cc= 0.900 n= 0.013 Clay tile

Primary OutFlow Max=0.77 cfs @ 12.45 hrs HW=253.72' (Free Discharge) ←1=Culvert (Inlet Controls 0.77 cfs @ 2.36 fps)

Type III 24-hr 10-yr Rainfall=4.40" Printed 3/16/2015 Page 13

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> Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: EX-1	Runoff Area=106,619 sf 31.99% Impervious Runoff Depth=0.85" Flow Length=887' Tc=17.1 min CN=58 Runoff=1.38 cfs 0.174 af
Subcatchment 2: EX-2	Runoff Area=82,355 sf 43.90% Impervious Runoff Depth=1.27"
Flow Lengt	h=580' Slope=0.0460 '/' Tc=9.9 min CN=65 Runoff=2.24 cfs 0.200 af
Subcatchment 3: EX-3	Runoff Area=26,342 sf 24.56% Impervious Runoff Depth=0.60" Flow Length=551' Tc=10.7 min CN=53 Runoff=0.22 cfs 0.030 af
Subcatchment 4: EX-4	Runoff Area=680 sf 11.65% Impervious Runoff Depth=0.31"
Flow Lengt	h=302' Slope=0.0500 '/' Tc=9.3 min CN=46 Runoff=0.00 cfs 0.000 af
Subcatchment 5: EX-5	Runoff Area=50,148 sf 21.62% Impervious Runoff Depth=0.55"
Flow Length	=561' Slope=0.0600 '/' Tc=11.1 min CN=52 Runoff=0.35 cfs 0.053 af
Subcatchment 6: EX-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=0.65" Flow Length=698' Tc=13.0 min CN=54 Runoff=1.28 cfs 0.175 af
Reach S1: north boundary swale n=0.035	Avg. Flow Depth=0.10' Max Vel=1.98 fps Inflow=1.38 cfs 0.174 af L=280.0' S=0.0571 '/' Capacity=117.19 cfs Outflow=1.37 cfs 0.174 af
Reach S2: south boundary swale n=0.035	Avg. Flow Depth=0.09' Max Vel=2.16 fps Inflow=2.24 cfs 0.200 af L=285.0' S=0.0702 '/' Capacity=403.00 cfs Outflow=2.17 cfs 0.200 af
Pond CB-1: CB-1 8.0"	Peak Elev=283.32' Inflow=0.35 cfs 0.053 af Round Culvert n=0.010 L=18.0' S=0.0506 '/' Outflow=0.35 cfs 0.053 af
Pond CB-2: CB-2	Peak Elev=283.28' Inflow=1.85 cfs 0.258 af
8.0" F	Round Culvert n=0.025 L=266.0' S=0.0650 '/' Outflow=1.85 cfs 0.258 af
Pond CB-3: CB-3	Peak Elev=283.35' Inflow=0.22 cfs 0.030 af
8.0"	Round Culvert n=0.025 L=60.0' S=0.0173 '/' Outflow=0.22 cfs 0.030 af
Pond DMH-1: DMH-1	Peak Elev=284.96' Inflow=1.28 cfs 0.175 af
8.0" F	Round Culvert n=0.013 L=136.0' S=0.0290 '/' Outflow=1.28 cfs 0.175 af
Pond DMH-2: DMH-2	Peak Elev=259.68' Inflow=5.04 cfs 0.632 af
10.0" F	Round Culvert n=0.013 L=200.0' S=0.0350 '/' Outflow=5.04 cfs 0.632 af
Total Runoff Area = 9	.341 ac Runoff Volume = 0.632 af Average Runoff Depth = 0.81"

69.81% Pervious = 6.521 ac 30.19% Impervious = 2.820 ac

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Type III 24-hr 25-yr Rainfall=5.00" Printed 3/16/2015 Page 21

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: EX-1	Runoff Area=106,619 sf 31.99% Impervious Runoff Depth=1.17" Flow Length=887' Tc=17.1 min CN=58 Runoff=2.06 cfs 0.238 af
Subcatchment 2: EX-2	Runoff Area=82,355 sf 43.90% Impervious Runoff Depth=1.65" / Length=580' Slope=0.0460 '/' Tc=9.9 min CN=65 Runoff=3.01 cfs 0.261 af
Subcatchment 3: EX-3	Runoff Area=26,342 sf 24.56% Impervious Runoff Depth=0.86" Flow Length=551' Tc=10.7 min CN=53 Runoff=0.38 cfs 0.043 af
Subcatchment 4: EX-4	Runoff Area=680 sf 11.65% Impervious Runoff Depth=0.49" / Length=302' Slope=0.0500 '/' Tc=9.3 min CN=46 Runoff=0.00 cfs 0.001 af
Subcatchment 5: EX-5 Flow	Runoff Area=50,148 sf 21.62% Impervious Runoff Depth=0.80" Length=561' Slope=0.0600 '/' Tc=11.1 min CN=52 Runoff=0.63 cfs 0.077 af
Subcatchment 6: EX-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=0.92" Flow Length=698' Tc=13.0 min CN=54 Runoff=2.11 cfs 0.248 af
Reach S1: north boundary swale	Avg. Flow Depth=0.12' Max Vel=2.27 fps Inflow=2.06 cfs 0.238 af 0.035 L=280.0' S=0.0571 '/' Capacity=117.19 cfs Outflow=2.04 cfs 0.238 af
Reach S2: south boundary swale	Avg. Flow Depth=0.11' Max Vel=2.40 fps Inflow=3.01 cfs 0.261 af 0.035 L=285.0' S=0.0702 '/' Capacity=403.00 cfs Outflow=2.93 cfs 0.261 af
Pond CB-1: CB-1	Peak Elev=340.15' Inflow=0.63 cfs 0.077 af 8.0" Round Culvert n=0.010 L=18.0' S=0.0506 '/' Outflow=0.63 cfs 0.077 af
Pond CB-2: CB-2	Peak Elev=340.03' Inflow=3.12 cfs 0.369 af 8.0" Round Culvert n=0.025 L=266.0' S=0.0650 '/' Outflow=3.12 cfs 0.369 af
Pond CB-3: CB-3	Peak Elev=340.21' Inflow=0.38 cfs 0.043 af 8.0" Round Culvert n=0.025 L=60.0' S=0.0173 '/' Outflow=0.38 cfs 0.043 af
Pond DMH-1: DMH-1	Peak Elev=344.42' Inflow=2.11 cfs 0.248 af 8.0" Round Culvert n=0.013 L=136.0' S=0.0290 '/' Outflow=2.11 cfs 0.248 af
Pond DMH-2: DMH-2	Peak Elev=276.45' Inflow=7.70 cfs 0.868 af 10.0" Round Culvert n=0.013 L=200.0' S=0.0350 '/' Outflow=7.70 cfs 0.868 af
Total Runoff Are	ea = 9.341 ac Runoff Volume = 0.868 af Average Runoff Depth = 1.11"

69.81% Pervious = 6.521 ac 30.19% Impervious = 2.820 ac

562801-Exist Allenstown Comm Center

Type III 24-hr 50-yr Rainfall=5.50" Printed 3/16/2015 Page 29

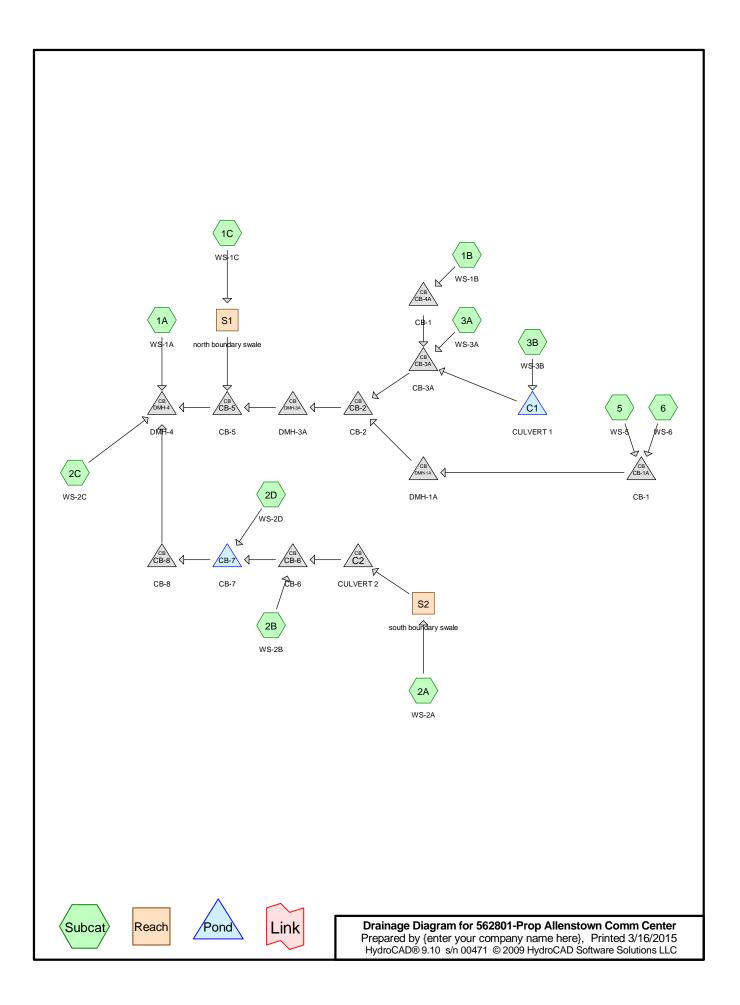
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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1: EX-1	Runoff Area=106,619 sf 31.99% Impervious Runoff Depth=1.45" Flow Length=887' Tc=17.1 min CN=58 Runoff=2.67 cfs 0.297 af
Subcatchment 2: EX-2	Runoff Area=82,355 sf 43.90% Impervious Runoff Depth=1.99" w Length=580' Slope=0.0460 '/' Tc=9.9 min CN=65 Runoff=3.70 cfs 0.314 af
Subcatchment 3: EX-3	Runoff Area=26,342 sf 24.56% Impervious Runoff Depth=1.10" Flow Length=551' Tc=10.7 min CN=53 Runoff=0.52 cfs 0.056 af
Subcatchment 4: EX-4	Runoff Area=680 sf 11.65% Impervious Runoff Depth=0.67" w Length=302' Slope=0.0500 '/' Tc=9.3 min CN=46 Runoff=0.01 cfs 0.001 af
Subcatchment 5: EX-5 Flow	Runoff Area=50,148 sf 21.62% Impervious Runoff Depth=1.04" Length=561' Slope=0.0600 '/' Tc=11.1 min CN=52 Runoff=0.90 cfs 0.099 af
Subcatchment 6: EX-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=1.17" Flow Length=698' Tc=13.0 min CN=54 Runoff=2.89 cfs 0.315 af
Reach S1: north boundary swale	Avg. Flow Depth=0.14' Max Vel=2.47 fps Inflow=2.67 cfs 0.297 af =0.035 L=280.0' S=0.0571 '/' Capacity=117.19 cfs Outflow=2.64 cfs 0.297 af
Reach S2: south boundary swale	Avg. Flow Depth=0.12' Max Vel=2.59 fps Inflow=3.70 cfs 0.314 af =0.035 L=285.0' S=0.0702 '/' Capacity=403.00 cfs Outflow=3.64 cfs 0.314 af
Pond CB-1: CB-1	Peak Elev=421.83' Inflow=0.90 cfs 0.099 af 8.0" Round Culvert n=0.010 L=18.0' S=0.0506 '/' Outflow=0.90 cfs 0.099 af
Pond CB-2: CB-2	Peak Elev=421.60' Inflow=4.31 cfs 0.471 af 8.0" Round Culvert n=0.025 L=266.0' S=0.0650 '/' Outflow=4.31 cfs 0.471 af
Pond CB-3: CB-3	Peak Elev=421.95' Inflow=0.52 cfs 0.056 af 8.0" Round Culvert n=0.025 L=60.0' S=0.0173 '/' Outflow=0.52 cfs 0.056 af
Pond DMH-1: DMH-1	Peak Elev=429.94' Inflow=2.89 cfs 0.315 af 8.0" Round Culvert n=0.013 L=136.0' S=0.0290 '/' Outflow=2.89 cfs 0.315 af
Pond DMH-2: DMH-2 1	Peak Elev=297.62' Inflow=10.09 cfs 1.082 af 0.0" Round Culvert n=0.013 L=200.0' S=0.0350 '/' Outflow=10.09 cfs 1.082 af
Total Runoff A	rea = 9.341 ac Runoff Volume = 1.082 af Average Runoff Depth = 1.39"

69.81% Pervious = 6.521 ac 30.19% Impervious = 2.820 ac

APPENDIX C



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# Area Listing (all nodes)

	Area	CN	Description	
(3	acres)		(subcatchment-numbers)	
	0.874	39	>75% Grass cover, Good, HSG A (1A, 1B, 1C, 2A, 2B, 2D, 3B)	
	0.819	46	2 acre lots, 12% imp, HSG A (3B)	
	0.935	51	1 acre lots, 20% imp, HSG A (1B)	
	3.957	54	1/2 acre lots, 25% imp, HSG A (5, 6)	
	1.742	61	1/4 acre lots, 38% imp, HSG A (1A, 2A)	
	0.834	98	Paved parking, HSG A (1B, 1C, 2A, 2B, 2C, 3A, 3B)	
	0.290	98	Paved roads w/curbs & sewers, HSG A (1A)	
	0.038	98	Roofs, HSG A (1C)	
	9.490		TOTAL AREA	

# Soil Listing (all nodes)

Soil	Subcatchment
Group	Numbers
HSG A	1A, 1B, 1C, 2A, 2B, 2C, 2D, 3A, 3B, 5, 6
HSG B	
HSG C	
HSG D	
Other	
	TOTAL AREA
	Group HSG A HSG B HSG C HSG D

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Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Fill (inches)
1	6	0.00	0.00	286.0	0.0400	0.013	8.0	0.0	0.0
2	C1	275.50	274.25	33.0	0.0379	0.010	18.0	0.0	0.0
3	C2	262.00	255.75	8.0	0.7813	0.010	18.0	0.0	0.0
4	CB-1A	275.75	274.53	122.0	0.0100	0.010	12.0	0.0	0.0
5	CB-2	270.90	265.48	175.0	0.0310	0.010	18.0	0.0	0.0
6	CB-3A	272.45	271.00	51.0	0.0284	0.010	18.0	0.0	0.0
7	CB-4A	273.75	272.55	119.5	0.0100	0.010	12.0	0.0	0.0
8	CB-5	253.65	253.25	18.0	0.0222	0.010	18.0	0.0	0.0
9	CB-6	255.65	255.16	49.0	0.0100	0.010	18.0	0.0	0.0
10	CB-7	255.06	254.24	82.0	0.0100	0.010	18.0	0.0	0.0
11	CB-8	253.24	253.00	42.0	0.0057	0.010	18.0	0.0	0.0
12	DMH-1A	274.06	273.89	17.5	0.0097	0.010	12.0	0.0	0.0
13	DMH-3A	270.90	253.75	82.0	0.2091	0.010	18.0	0.0	0.0
14	DMH-4	253.00	247.00	200.0	0.0300	0.013	10.0	0.0	0.0

# Pipe Listing (all nodes)

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#### Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1A: WS-1A	Runoff Area=45,731 sf 54.20% Impervious Runoff Depth=0.70" Flow Length=508' Tc=9.6 min CN=71 Runoff=0.66 cfs 0.062 af
Subcatchment 1B: WS-1B	Runoff Area=43,860 sf 24.41% Impervious Runoff Depth=0.15" Flow Length=607' Tc=14.1 min CN=54 Runoff=0.04 cfs 0.012 af
Subcatchment 1C: WS-1C	Runoff Area=27,533 sf 32.94% Impervious Runoff Depth=0.24" Flow Length=351' Tc=9.0 min CN=58 Runoff=0.06 cfs 0.013 af
Subcatchment 2A: WS-2A	Runoff Area=70,105 sf 48.25% Impervious Runoff Depth=0.54" w Length=466' Slope=0.0460 '/' Tc=11.5 min CN=67 Runoff=0.64 cfs 0.072 af
Subcatchment 2B: WS-2B F	Runoff Area=2,611 sf 11.18% Impervious Runoff Depth=0.02" low Length=132' Slope=0.0250 '/' Tc=9.9 min CN=46 Runoff=0.00 cfs 0.000 af
Subcatchment 2C: WS-2C	Runoff Area=4,493 sf 100.00% Impervious Runoff Depth=2.67" Flow Length=267' Tc=1.7 min CN=98 Runoff=0.31 cfs 0.023 af
Subcatchment 2D: WS-2D	Runoff Area=2,894 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=64' Slope=0.2000 '/' Tc=2.9 min CN=39 Runoff=0.00 cfs 0.000 af
Subcatchment 3A: WS-3A	Runoff Area=3,800 sf 100.00% Impervious Runoff Depth=2.67" Flow Length=181' Tc=1.7 min CN=98 Runoff=0.27 cfs 0.019 af
Subcatchment 3B: WS-3B	Runoff Area=39,994 sf 12.31% Impervious Runoff Depth=0.02" Flow Length=585' Tc=14.0 min CN=46 Runoff=0.00 cfs 0.002 af
Subcatchment 5: WS-5	Runoff Area=31,606 sf 25.00% Impervious Runoff Depth=0.15" Flow Length=385' Tc=9.6 min CN=54 Runoff=0.03 cfs 0.009 af
Subcatchment 6: WS-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=0.15" Flow Length=698' Tc=13.0 min CN=54 Runoff=0.12 cfs 0.040 af
Reach S1: north boundary swal	e Avg. Flow Depth=0.02' Max Vel=0.89 fps Inflow=0.06 cfs 0.013 af n=0.030 L=256.0' S=0.0684 '/' Capacity=69.96 cfs Outflow=0.06 cfs 0.013 af
Reach S2: south boundary swa	Avg. Flow Depth=0.07' Max Vel=2.01 fps Inflow=0.64 cfs 0.072 af n=0.030 L=266.0' S=0.0583 '/' Capacity=64.60 cfs Outflow=0.63 cfs 0.072 af
Pond C1: CULVERT 1	Peak Elev=276.00' Storage=0 cf Inflow=0.00 cfs 0.002 af 18.0" Round Culvert n=0.010 L=33.0' S=0.0379 '/' Outflow=0.00 cfs 0.002 af
Pond C2: CULVERT 2	Peak Elev=262.35' Inflow=0.63 cfs 0.072 af 18.0" Round Culvert n=0.010 L=8.0' S=0.7813 '/' Outflow=0.63 cfs 0.072 af
Pond CB-1A: CB-1	Peak Elev=275.93' Inflow=0.14 cfs 0.049 af 12.0" Round Culvert n=0.010 L=122.0' S=0.0100 '/' Outflow=0.14 cfs 0.049 af

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Pond CB-2: CB-2	Peak Elev=271.27' Inflow=0.27 cfs 0.082 af 18.0" Round Culvert n=0.010 L=175.0' S=0.0310 '/' Outflow=0.27 cfs 0.082 af
Pond CB-3A: CB-3A	Peak Elev=272.67' Inflow=0.27 cfs 0.034 af 18.0" Round Culvert n=0.010 L=51.0' S=0.0284 '/' Outflow=0.27 cfs 0.034 af
Pond CB-4A: CB-1	Peak Elev=273.84' Inflow=0.04 cfs 0.012 af 12.0" Round Culvert n=0.010 L=119.5' S=0.0100 '/' Outflow=0.04 cfs 0.012 af
Pond CB-5: CB-5	Peak Elev=253.88' Inflow=0.27 cfs 0.095 af 18.0" Round Culvert n=0.010 L=18.0' S=0.0222 '/' Outflow=0.27 cfs 0.095 af
Pond CB-6: CB-6	Peak Elev=259.51' Inflow=0.63 cfs 0.072 af 18.0" Round Culvert n=0.010 L=49.0' S=0.0100 '/' Outflow=0.63 cfs 0.072 af
Pond CB-7: CB-7	Peak Elev=259.50' Storage=0 cf Inflow=0.63 cfs 0.072 af 18.0" Round Culvert n=0.010 L=82.0' S=0.0100 '/' Outflow=0.63 cfs 0.072 af
Pond CB-8: CB-8	Peak Elev=253.81' Inflow=0.63 cfs 0.072 af 18.0" Round Culvert n=0.010 L=42.0' S=0.0057 '/' Outflow=0.63 cfs 0.072 af
Pond DMH-1A: DMH-1A	Peak Elev=274.24' Inflow=0.14 cfs 0.049 af 12.0" Round Culvert n=0.010 L=17.5' S=0.0097 '/' Outflow=0.14 cfs 0.049 af
Pond DMH-3A: DMH-3A	Peak Elev=271.12' Inflow=0.27 cfs 0.082 af 18.0" Round Culvert n=0.010 L=82.0' S=0.2091 '/' Outflow=0.27 cfs 0.082 af
Pond DMH-4: DMH-4	Peak Elev=253.73' Inflow=1.48 cfs 0.251 af 10.0" Round Culvert n=0.013 L=200.0' S=0.0300 '/' Outflow=1.48 cfs 0.251 af
Total Runoff	Area = 9.490 ac Runoff Volume = 0.252 af Average Runoff Depth = 0.32" 67.35% Pervious = 6.391 ac 32.65% Impervious = 3.099 ac

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## Summary for Subcatchment 1A: WS-1A

Runoff = 0.66 cfs @ 12.16 hrs, Volume= 0.062 af, Depth= 0.70"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

_	A	rea (sf)	CN I	Description					
		31,245	61 <sup>·</sup>	1/4 acre lots, 38% imp, HSG A					
		12,634	98 I	Paved road	s w/curbs &	& sewers, HSG A			
		738			s, 38% imp				
_		1,114	39 :	-75% Gras	s cover, Go	ood, HSG A			
		45,731		Veighted A	•				
		20,943			vious Area				
		24,788	Ę	54.20% Imp	pervious Ar	ea			
	-		~		<b>•</b> •				
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.2	100	0.0700	0.27		Sheet Flow,			
						Grass: Short n= 0.150 P2= 2.90"			
	3.0	331	0.0700	1.85		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.4	77	0.0200	2.87		Shallow Concentrated Flow,			
_						Paved Kv= 20.3 fps			
	9.6	508	Total						

9.6 508 Total

### Summary for Subcatchment 1B: WS-1B

Runoff = 0.04 cfs @ 12.55 hrs, Volume= 0.012 af, Depth= 0.15"

A	rea (sf)	CN	Description					
	40,745	51	1 acre lots, 20% imp, HSG A					
	558	39 :	>75% Gras	s cover, Go	bod, HSG A			
	2,557	98	Paved park	ing, HSG A	١			
	43,860	54	Weighted A	verage				
	33,154		75.59% Per	vious Area				
	10,706		24.41% Imp	pervious Ar	ea			
_				•				
Тс	Length	Slope	•	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
7.1	100	0.0500	0.23		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.90"			
6.6	445	0.0500	1.12		Shallow Concentrated Flow,			
					Woodland Kv= 5.0 fps			
0.4	62	0.0200	2.87		Shallow Concentrated Flow,			
					Paved Kv= 20.3 fps			
14.1	607	Total						

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## Summary for Subcatchment 1C: WS-1C

Runoff = 0.06 cfs @ 12.37 hrs, Volume= 0.013 af, Depth= 0.24"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

	Ar	ea (sf)	CN	Description				
		7,420	98	98 Paved parking, HSG A				
		1,648	98	Roofs, HSC	<b>S</b> A			
		18,465	39	>75% Gras	s cover, Go	bod, HSG A		
	2	27,533	58	Weighted A	verage			
		18,465		67.06% Pei	vious Area			
		9,068		32.94% lmp	pervious Are	ea		
-	Гс	Length	Slope			Description		
(mi	n)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
7	.1	100	0.0500	0.23		Sheet Flow,		
						Grass: Short n= 0.150 P2= 2.90"		
1	.2	105	0.0400	1.40		Shallow Concentrated Flow,		
						Short Grass Pasture Kv= 7.0 fps		
0	.6	146	0.0700	3.97		Shallow Concentrated Flow,		
						Grassed Waterway Kv= 15.0 fps		
9	.0	351	Total					

# Summary for Subcatchment 2A: WS-2A

Runoff = 0.64 cfs @ 12.20 hrs, Volume= 0.072 af, Depth= 0.54"

A	rea (sf)	CN	Description					
	43,883	61	1/4 acre lots	I/4 acre lots, 38% imp, HSG A				
	17,148	98	Paved park	ing, HSG A	N Contraction of the second			
	9,074	39	>75% Gras	s cover, Go	bod, HSG A			
	70,105	67	Weighted A	verage				
	36,281		51.75% Pei	vious Area				
	33,824		48.25% Imp	pervious Ar	ea			
Tc	Length	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)				
7.4	100	0.046	0.23		Sheet Flow,			
					Grass: Short n= 0.150 P2= 2.90"			
4.1	366	0.046	0 1.50		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
11.5	466	Total						

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### Summary for Subcatchment 2B: WS-2B

Runoff = 0.00 cfs @ 16.93 hrs, Volume= 0.000 af, Depth= 0.02"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

Δ	rea (sf)	CN D	escription					
	292							
	2,319							
	2,611		Veighted A					
	2,319	-		vious Area				
	292	1	1.18% Imp	pervious Ar	ea			
-				<b>•</b> •				
	0				Description			
<u>(min)</u> 9.4	(feet)	(ft/ft)	(ft/sec) 0.18	(cfs)	Sheet Flow			
9.4	100	0.0250	0.18		Sheet Flow, Grass: Short n= 0.150 P2= 2.90"			
0.5	32	0.0250	1.11		Shallow Concentrated Flow,			
0.0		0.0200			Short Grass Pasture Kv= 7.0 fps			
9.9	132	Total						
			Sum	mary for S	Subcatchment 2C: WS-2C			
Runoff	=	0.31 cf	s@ 12.03	3 hrs, Volu	me= 0.023 af, Depth= 2.67"			
Runoff b	y SCS TF	R-20 met	hod, UH=S		me= 0.023 af, Depth= 2.67" Span= 0.00-48.00 hrs, dt= 0.05 hrs			
Runoff b		R-20 met	hod, UH=S					
Runoff b Type III :	y SCS TF	R-20 met <sup>-</sup> Rainfall	hod, UH=S					
Runoff b Type III :	y SCS TF 24-hr 2-yr	R-20 met <sup>r</sup> Rainfall <u>CN D</u>	hod, UH=S =2.90" Description		Span= 0.00-48.00 hrs, dt= 0.05 hrs			
Runoff b Type III :	by SCS TF 24-hr 2-yr rea (sf)	R-20 met <sup>-</sup> Rainfall <u>CN D</u> 98 P	hod, UH=S =2.90" Pescription Paved park	SCS, Time S	Span= 0.00-48.00 hrs, dt= 0.05 hrs			
Runoff b Type III : A	oy SCS TF 24-hr 2-yr <u>rea (sf)</u> <u>4,493</u> 4,493	R-20 met Rainfall <u>CN D</u> 98 P 1	hod, UH=S =2.90" Pescription Paved park 00.00% Im	SCS, Time S ing, HSG A apervious A	Span= 0.00-48.00 hrs, dt= 0.05 hrs rea			
Runoff b Type III 2 A A	by SCS TF 24-hr 2-yr <u>rea (sf) 4,493</u> 4,493 Length	R-20 met Rainfall <u>CN D</u> 98 F 1 Slope	hod, UH=S =2.90" Pescription Paved park 00.00% Im Velocity	SCS, Time S ing, HSG A pervious A Capacity	Span= 0.00-48.00 hrs, dt= 0.05 hrs			
Runoff b Type III : A 	by SCS TF 24-hr 2-yr <u>rea (sf)</u> 4,493 4,493 Length (feet)	R-20 met Rainfall <u>CN D</u> 98 P 1 Slope (ft/ft)	hod, UH=S =2.90" Pescription Paved park 00.00% Im Velocity (ft/sec)	SCS, Time S ing, HSG A apervious A	Span= 0.00-48.00 hrs, dt= 0.05 hrs rea Description			
Runoff b Type III 2 A A	by SCS TF 24-hr 2-yr <u>rea (sf) 4,493</u> 4,493 Length	R-20 met Rainfall <u>CN D</u> 98 F 1 Slope	hod, UH=S =2.90" Pescription Paved park 00.00% Im Velocity	SCS, Time S ing, HSG A pervious A Capacity	Span= 0.00-48.00 hrs, dt= 0.05 hrs rea Description <b>Sheet Flow,</b>			
Runoff b Type III : A 	by SCS TF 24-hr 2-yr <u>rea (sf) 4,493</u> 4,493 Length (feet) 20	R-20 met Rainfall <u>CN E</u> 98 F 1 Slope (ft/ft) 0.0200	hod, UH=S =2.90" Pescription Paved park 00.00% Im Velocity (ft/sec) 0.95	SCS, Time S ing, HSG A pervious A Capacity	Span= 0.00-48.00 hrs, dt= 0.05 hrs rea Description <b>Sheet Flow,</b> Smooth surfaces n= 0.011 P2= 2.90"			
Runoff b Type III : A 	by SCS TF 24-hr 2-yr <u>rea (sf)</u> 4,493 4,493 Length (feet)	R-20 met Rainfall <u>CN D</u> 98 P 1 Slope (ft/ft)	hod, UH=S =2.90" Pescription Paved park 00.00% Im Velocity (ft/sec)	SCS, Time S ing, HSG A pervious A Capacity	Span= 0.00-48.00 hrs, dt= 0.05 hrs rea Description Sheet Flow, Smooth surfaces n= 0.011 P2= 2.90" Shallow Concentrated Flow,			
Runoff b Type III : A 	by SCS TF 24-hr 2-yr <u>rea (sf)</u> 4,493 4,493 Length (feet) 20 247	R-20 met Rainfall <u>CN E</u> 98 F 1 Slope (ft/ft) 0.0200	hod, UH=S =2.90" Pescription Paved park 00.00% Im Velocity (ft/sec) 0.95	SCS, Time S ing, HSG A pervious A Capacity	Span= 0.00-48.00 hrs, dt= 0.05 hrs rea Description <b>Sheet Flow,</b> Smooth surfaces n= 0.011 P2= 2.90"			

### Summary for Subcatchment 2D: WS-2D

Runoff = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Depth= 0.00"

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A	rea (sf)		escription				
	2,894	39 >	75% Gras	s cover, Go	bod, HSG A		
	2,894	1	00.00% Pe	ervious Are	a		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
2.9	64	0.2000	0.37		Sheet Flow,		
					Grass: Short n= 0.150 P2= 2.90"		
			Sum	mary for 9	Subcatchment 3A: WS-3A		
			Sum		Juscalenment JA. WO-JA		
Runoff	=	0.27 cfs	s@ 12.0	3 hrs, Volu	me= 0.019 af, Depth= 2.67"		
	y SCS TF 24-hr 2-yr			SCS, Time S	Span= 0.00-48.00 hrs, dt= 0.05 hrs		
A	rea (sf)	CN D	escription				
	3,800	98 P	aved park	ing, HSG A	۱		
	3,800	1	00.00% In	npervious A	rea		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
0.4	20	0.0200	0.95		Sheet Flow,		
					Smooth surfaces $n=0.011$ P2= 2.90"		
1.0	118	0.0100	2.03		Shallow Concentrated Flow,		
0.3	43	0.0200	2.12		Paved Kv= 20.3 fps Shallow Concentrated Flow,		
0.0	-10	0.0200	2.12		Grassed Waterway Kv= 15.0 fps		
1.7	181	Total			· · · · ·		
	Summary for Subcatchment 3B: WS-3B						

Runoff = 0.00 cfs @ 16.97 hrs, Volume= 0.002 af, Depth= 0.02"

Area (sf)	CN	Description
35,687	46	2 acre lots, 12% imp, HSG A
3,667	39	>75% Grass cover, Good, HSG A
640	98	Paved parking, HSG A
39,994	46	Weighted Average
35,072		87.69% Pervious Area
4,922		12.31% Impervious Area

Type III 24-hr 2-yr Rainfall=2.90" Printed 3/16/2015 Page 11

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	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	7.1	100	0.0500	0.23		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.90"
	6.6	442	0.0500	1.12		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.3	43	0.0200	2.12		Shallow Concentrated Flow,
_						Grassed Waterway Kv= 15.0 fps
_	14.0	585	Total			

#### Summary for Subcatchment 5: WS-5

Runoff = 0.03 cfs @ 12.48 hrs, Volume= 0.009 af, Depth= 0.15"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Type III 24-hr 2-yr Rainfall=2.90"

_	Ai	rea (sf)	CN D	<b>Description</b>		
		31,606	54 1	/2 acre lots	s, 25% imp	, HSG A
		23,705	7	5.00% Per	vious Area	
		7,902	2	5.00% Imp	pervious Are	ea
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
-	6.6	100	0.0600	0.25	<u> </u>	Sheet Flow,
	3.0	285	0.0500	1.57		Grass: Short n= 0.150 P2= 2.90" <b>Shallow Concentrated Flow,</b> Short Grass Pasture Kv= 7.0 fps
	9.6	385	Total			

### Summary for Subcatchment 6: WS-6

Runoff = 0.12 cfs @ 12.53 hrs, Volume= 0.040 af, Depth= 0.15"

Area (sf)	CN	Description
140,742	54	1/2 acre lots, 25% imp, HSG A
105,557 35,186		75.00% Pervious Area 25.00% Impervious Area

Type III 24-hr 2-yr Rainfall=2.90" Printed 3/16/2015 Page 12

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_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	8.0	100	0.0380	0.21		Sheet Flow,
						Grass: Short n= 0.150 P2= 2.90"
	4.3	312	0.0300	1.21		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.7	286	0.0400	6.92	2.42	Pipe Channel, Offsite Piped Flow from East
						8.0" Round Area= 0.3 sf Perim= 2.1' r= 0.17'
_						n= 0.013 Clay tile

13.0 698 Total

### Summary for Reach S1: north boundary swale

Inflow Area =	0.632 ac, 32.94% Impervious, Inflow E	Depth = 0.24" for 2-yr event
Inflow =	0.06 cfs @ 12.37 hrs, Volume=	0.013 af
Outflow =	0.06 cfs @ 12.43 hrs, Volume=	0.013 af, Atten= 3%, Lag= 3.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 0.89 fps, Min. Travel Time= 4.8 min Avg. Velocity = 0.61 fps, Avg. Travel Time= 7.0 min

Peak Storage= 18 cf @ 12.43 hrs Average Depth at Peak Storage= 0.02' Defined Flood Depth= 1.50', Capacity at Flood Depth= 138.59 cfs Bank-Full Depth= 1.00', Capacity at Bank-Full= 69.96 cfs

4.00' x 1.00' deep channel, n= 0.030 Earth, dense weeds Side Slope Z-value= 3.0 '/' Top Width= 10.00' Length= 256.0' Slope= 0.0684 '/' Inlet Invert= 277.00', Outlet Invert= 259.50'



### Summary for Reach S2: south boundary swale

Inflow Area =	1.609 ac, 48.25% Impervious, Inflow	Depth = 0.54" for 2-yr event
Inflow =	0.64 cfs @ 12.20 hrs, Volume=	0.072 af
Outflow =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af, Atten= 1%, Lag= 1.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Max. Velocity= 2.01 fps, Min. Travel Time= 2.2 min Avg. Velocity = 0.78 fps, Avg. Travel Time= 5.7 min

Peak Storage= 83 cf @ 12.22 hrs Average Depth at Peak Storage= 0.07' Defined Flood Depth= 1.50', Capacity at Flood Depth= 127.96 cfs

Type III 24-hr 2-yr Rainfall=2.90" Printed 3/16/2015 Page 13

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Bank-Full Depth= 1.00', Capacity at Bank-Full= 64.60 cfs

4.00' x 1.00' deep channel, n= 0.030 Earth, dense weeds Side Slope Z-value= 3.0 '/' Top Width= 10.00' Length= 266.0' Slope= 0.0583 '/' Inlet Invert= 279.00', Outlet Invert= 263.50'

‡

## Summary for Pond C1: CULVERT 1

Inflow Area =	0.918 ac, 12.31% Impervious, Inflow I	Depth = 0.02" for 2-yr event
Inflow =	0.00 cfs @ 16.97 hrs, Volume=	0.002 af
Outflow =	0.00 cfs @ 16.97 hrs, Volume=	0.002 af, Atten= 0%, Lag= 0.0 min
Primary =	0.00 cfs @ 16.97 hrs, Volume=	0.002 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 276.00' @ 0.00 hrs Surf.Area= 25 sf Storage= 0 cf Flood Elev= 278.00' Surf.Area= 502 sf Storage= 527 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 0.0 min (1,157.4 - 1,157.4)

Volume	Inv	ert Avail.Sto	orage Storage I	Description	
#1	276.	00' 5	27 cf Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Elevatio (fee 276.0 278.0	et) 00	Surf.Area (sq-ft) 25 502	Inc.Store (cubic-feet) 0 527	Cum.Store (cubic-feet) 0 527	
Device	Routing	Invert	Outlet Devices	i	
#1	Primary	275.50'	Inlet / Outlet In	, square edge h	neadwall, Ke= 0.500 274.25' S= 0.0379 '/' Cc= 0.900 or

Primary OutFlow Max=0.00 cfs @ 16.97 hrs HW=276.00' TW=272.51' (Dynamic Tailwater) -1=Culvert (Passes 0.00 cfs of 1.24 cfs potential flow)

## Summary for Pond C2: CULVERT 2

Inflow A Inflow Outflow Primary	=	0.63 cfs @ 12 0.63 cfs @ 12	25% Impervious, Inflow Depth =       0.54" for 2-yr event         2.22 hrs, Volume=       0.072 af         2.22 hrs, Volume=       0.072 af, Atten= 0%, Lag= 0.0 min         2.22 hrs, Volume=       0.072 af		
Peak El	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 262.35' @ 12.22 hrs Flood Elev= 264.00'				
Device	Routing	Invert	Outlet Devices		
#1	Primary	262.00'	<b>18.0"</b> Round Culvert L= 8.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= $262.00' / 255.75'$ S= 0.7813 '/' Cc= 0.900 n= 0.010 PVC, smooth interior		

Primary OutFlow Max=0.62 cfs @ 12.22 hrs HW=262.35' TW=259.51' (Dynamic Tailwater)

## Summary for Pond CB-1A: CB-1

Inflow Area	=	3.957 ac, 25.0	0% Impervious, Inflow D	Depth = 0.15" for 2-yr event
Inflow	=	0.14 cfs @ 12.	.52 hrs, Volume=	0.049 af
Outflow	=	0.14 cfs @ 12.	.52 hrs, Volume=	0.049 af, Atten= 0%, Lag= 0.0 min
Primary	=	0.14 cfs @ 12.	.52 hrs, Volume=	0.049 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 275.93' @ 12.52 hrs Flood Elev= 281.00'

	Device	ce Routing	Invert	Outlet Devices
#1 Primary 275.75' <b>12.0" Round Culvert</b> L= 122.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 275.75' / 274.53' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior	-	0	275.75'	L= 122.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 275.75' / 274.53' S= 0.0100 '/' Cc= 0.900

Primary OutFlow Max=0.14 cfs @ 12.52 hrs HW=275.93' TW=274.24' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.14 cfs @ 1.45 fps)

## Summary for Pond CB-2: CB-2

Inflow Area	a =	5.969 ac, 2	4.04% Impervious	, Inflow Depth =	0.17" for 2-yr event
Inflow	=	0.27 cfs @	12.03 hrs, Volum	e= 0.082	af
Outflow	=	0.27 cfs @	12.03 hrs, Volum	e= 0.082	af, Atten= 0%, Lag= 0.0 min
Primary	=	0.27 cfs @	12.03 hrs, Volum	e= 0.082	af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 271.27' @ 12.04 hrs Flood Elev= 276.75'

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Device	Routing	Invert	Outlet Devices
#1	Primary	270.90'	<b>18.0" Round Culvert</b> L= 175.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 270.90' / 265.48' S= 0.0310 '/' Cc= 0.900 n= 0.010 PVC, smooth interior
			2 12.03 hrs HW=271.26' TW=271.12' (Dynamic Tailwater)

**1=Culvert** (Outlet Controls 0.24 cfs @ 1.11 fps)

### Summary for Pond CB-3A: CB-3A

Inflow Area =	2.012 ac, 22.16% Impervious, Inflow Depth = 0.20"	for 2-yr event
Inflow =	0.27 cfs @ 12.03 hrs, Volume= 0.034 af	
Outflow =	0.27 cfs @ 12.03 hrs, Volume= 0.034 af, Atte	en= 0%, Lag= 0.0 min
Primary =	0.27 cfs @ 12.03 hrs, Volume= 0.034 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 272.67' @ 12.03 hrs Flood Elev= 277.50'

DeviceRoutingInvertOutlet Devices#1Primary272.45'**18.0" Round Culvert** L= 51.0' RCP, sq.cut end projecting, Ke= 0.500<br/>Inlet / Outlet Invert= 272.45' / 271.00' S= 0.0284 '/' Cc= 0.900<br/>n= 0.010 PVC, smooth interior

Primary OutFlow Max=0.25 cfs @ 12.03 hrs HW=272.67' TW=271.26' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 0.25 cfs @ 1.59 fps)

## Summary for Pond CB-4A: CB-1

Inflow Area =	=	1.007 ac, 2	4.41% Impervious,	Inflow Depth = 0.1	5" for 2-yr event
Inflow =		0.04 cfs @	12.55 hrs, Volume	e= 0.012 af	
Outflow =		0.04 cfs @	12.55 hrs, Volume	e= 0.012 af,	Atten= 0%, Lag= 0.0 min
Primary =		0.04 cfs @	12.55 hrs, Volume	e= 0.012 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 273.84' @ 12.55 hrs Flood Elev= 279.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.75'	<b>12.0" Round Culvert</b> L= 119.5' RCP, sq.cut end projecting, Ke= $0.500$ Inlet / Outlet Invert= 273.75' / 272.55' S= $0.0100$ '/' Cc= $0.900$ n= $0.010$ PVC, smooth interior

Primary OutFlow Max=0.04 cfs @ 12.55 hrs HW=273.84' TW=272.56' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.04 cfs @ 1.02 fps) Prepared by {enter your company name here} HydroCAD® 9.10 s/n 00471 © 2009 HydroCAD Software Solutions LLC

# Summary for Pond CB-5: CB-5

Inflow A Inflow Outflow Primary	=	0.27 cfs @ 12 0.27 cfs @ 12	90% Impervious, Inflow Depth =       0.17" for 2-yr event         2.48 hrs, Volume=       0.095 af         2.48 hrs, Volume=       0.095 af, Atten= 0%, Lag= 0.0 min         2.48 hrs, Volume=       0.095 af		
Peak El	Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 253.88' @ 12.48 hrs Flood Elev= 259.50'				
Device	Routing	Invert	Outlet Devices		
#1	Primary	253.65'	<b>18.0"</b> Round Culvert L= 18.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= $253.65' / 253.25' S= 0.0222'/' Cc= 0.900$ n= 0.010 PVC, smooth interior		

Primary OutFlow Max=0.27 cfs @ 12.48 hrs HW=253.88' TW=253.58' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 0.27 cfs @ 1.62 fps)

# Summary for Pond CB-6: CB-6

Inflow Area =	1.669 ac, 46.92% Impervious, Inflow I	Depth = 0.52" for 2-yr event
Inflow =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af
Outflow =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af, Atten= 0%, Lag= 0.0 min
Primary =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 259.51' @ 12.22 hrs Flood Elev= 261.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	255.65'	<b>18.0" Round Culvert</b> L= 49.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 255.65' / 255.16' S= 0.0100 '/' Cc= 0.900 n= 0.010 PVC, smooth interior

Primary OutFlow Max=0.62 cfs @ 12.22 hrs HW=259.51' TW=259.50' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.62 cfs @ 0.35 fps)

## Summary for Pond CB-7: CB-7

Inflow Area =	1.736 ac,	45.12% Impervious, Inf	flow Depth = 0.50" for 2-yr event
Inflow =	0.63 cfs @	2 12.22 hrs, Volume=	0.072 af
Outflow =	0.63 cfs @	2 12.22 hrs, Volume=	0.072 af, Atten= 0%, Lag= 0.0 min
Primary =	0.63 cfs @	2 12.22 hrs, Volume=	0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 259.50' @ 0.00 hrs Surf.Area= 15 sf Storage= 0 cf Flood Elev= 260.00' Surf.Area= 318 sf Storage= 83 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 0.0 min ( 906.5 - 906.5 )

Volume	Inve	ert Avail.St	orage	Storage	Description	
#1	259.5	50'	83 cf	Custom	Stage Data (Pri	smatic) Listed below (Recalc)
Elevatio	et)	Surf.Area (sq-ft)	Inc. (cubic	/	Cum.Store (cubic-feet)	
259.5	-	15		0	0	
260.0	)0	318		83	83	
Device #1	Routing Primary	Invert 255.06	18.0"	/ Outlet I	Culvert L= 82.	0' RCP, sq.cut end projecting, Ke= 0.500 254.24' S= 0.0100 '/' Cc= 0.900 or

Primary OutFlow Max=0.00 cfs @ 12.22 hrs HW=259.50' TW=253.80' (Dynamic Tailwater) ←1=Culvert (Passes 0.00 cfs of 16.34 cfs potential flow)

### Summary for Pond CB-8: CB-8

Inflow Area =	1.736 ac, 45.12% Impervious, Inflow E	Depth = 0.50" for 2-yr event
Inflow =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af
Outflow =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af, Atten= 0%, Lag= 0.0 min
Primary =	0.63 cfs @ 12.22 hrs, Volume=	0.072 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 253.81' @ 12.24 hrs Flood Elev= 258.25'

 Device
 Routing
 Invert
 Outlet Devices

 #1
 Primary
 253.24'
 **18.0" Round Culvert** L= 42.0' RCP, sq.cut end projecting, Ke= 0.500

 Inlet / Outlet Invert=
 253.24' / 253.00' S= 0.0057 '/' Cc= 0.900
 n= 0.010 PVC, smooth interior

Primary OutFlow Max=0.64 cfs @ 12.22 hrs HW=253.80' TW=253.72' (Dynamic Tailwater) ←1=Culvert (Outlet Controls 0.64 cfs @ 1.55 fps)

### Summary for Pond DMH-1A: DMH-1A

Inflow Area	a =	3.957 ac, 2	25.00% Impervious, I	Inflow Depth = 0.15" for 2-yr event
Inflow	=	0.14 cfs @	12.52 hrs, Volume=	= 0.049 af
Outflow	=	0.14 cfs @	12.52 hrs, Volume=	= 0.049 af, Atten= 0%, Lag= 0.0 min
Primary	=	0.14 cfs @	12.52 hrs, Volume=	= 0.049 af

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 274.24' @ 12.52 hrs Flood Elev= 277.25'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.06'	<b>12.0" Round Culvert</b> L= 17.5' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 274.06' / 273.89' S= 0.0097 '/' Cc= 0.900 n= 0.010 PVC, smooth interior

Primary OutFlow Max=0.14 cfs @ 12.52 hrs HW=274.24' TW=271.24' (Dynamic Tailwater) -1=Culvert (Barrel Controls 0.14 cfs @ 2.20 fps)

### Summary for Pond DMH-3A: DMH-3A

Inflow Area	ι =	5.969 ac, 2	24.04% Impervious, Inflo	by Depth = $0.17$ "	for 2-yr event
Inflow	=	0.27 cfs @	12.03 hrs, Volume=	0.082 af	
Outflow	=	0.27 cfs @	12.03 hrs, Volume=	0.082 af, Atte	en= 0%, Lag= 0.0 min
Primary	=	0.27 cfs @	12.03 hrs, Volume=	0.082 af	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 271.12' @ 12.03 hrs Flood Elev= 271.23'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.90'	<b>18.0" Round Culvert</b> L= 82.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 270.90' / 253.75' S= 0.2091 '/' Cc= 0.900 n= 0.010 PVC, smooth interior

Primary OutFlow Max=0.25 cfs @ 12.03 hrs HW=271.12' TW=253.87' (Dynamic Tailwater) ←1=Culvert (Inlet Controls 0.25 cfs @ 1.59 fps)

### Summary for Pond DMH-4: DMH-4

Inflow Area	=	9.490 ac, 3	32.65% Impervious,	Inflow Depth = (	0.32" for 2-yr event
Inflow =	=	1.48 cfs @	12.20 hrs, Volume	e= 0.251 a	f
Outflow =	=	1.48 cfs @	12.20 hrs, Volume	e= 0.251 a	f, Atten= 0%, Lag= 0.0 min
Primary =	=	1.48 cfs @	12.20 hrs, Volume	e= 0.251 at	f

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 253.73' @ 12.20 hrs Flood Elev= 259.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	253.00'	<b>10.0" Round Culvert</b> L= 200.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 253.00' / 247.00' S= 0.0300 '/' Cc= 0.900 n= 0.013 Clay tile

Primary OutFlow Max=1.47 cfs @ 12.20 hrs HW=253.73' (Free Discharge) -1=Culvert (Inlet Controls 1.47 cfs @ 2.91 fps)

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#### Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1A: WS-1A	Runoff Area=45,731 sf 54.20% Impervious Runoff Depth=1.67" Flow Length=508' Tc=9.6 min CN=71 Runoff=1.75 cfs 0.146 af
Subcatchment 1B: WS-1B	Runoff Area=43,860 sf 24.41% Impervious Runoff Depth=0.65" Flow Length=607' Tc=14.1 min CN=54 Runoff=0.39 cfs 0.054 af
Subcatchment 1C: WS-1C	Runoff Area=27,533 sf 32.94% Impervious Runoff Depth=0.85" Flow Length=351' Tc=9.0 min CN=58 Runoff=0.45 cfs 0.045 af
Subcatchment 2A: WS-2A Flo	Runoff Area=70,105 sf 48.25% Impervious Runoff Depth=1.40" w Length=466' Slope=0.0460 '/' Tc=11.5 min CN=67 Runoff=2.05 cfs 0.188 af
Subcatchment 2B: WS-2B	Runoff Area=2,611 sf 11.18% Impervious Runoff Depth=0.31" low Length=132' Slope=0.0250 '/' Tc=9.9 min CN=46 Runoff=0.01 cfs 0.002 af
Subcatchment 2C: WS-2C	Runoff Area=4,493 sf 100.00% Impervious Runoff Depth=4.16" Flow Length=267' Tc=1.7 min CN=98 Runoff=0.48 cfs 0.036 af
Subcatchment 2D: WS-2D	Runoff Area=2,894 sf 0.00% Impervious Runoff Depth=0.10" Flow Length=64' Slope=0.2000 '/' Tc=2.9 min CN=39 Runoff=0.00 cfs 0.001 af
Subcatchment 3A: WS-3A	Runoff Area=3,800 sf 100.00% Impervious Runoff Depth=4.16" Flow Length=181' Tc=1.7 min CN=98 Runoff=0.41 cfs 0.030 af
Subcatchment 3B: WS-3B	Runoff Area=39,994 sf 12.31% Impervious Runoff Depth=0.31" Flow Length=585' Tc=14.0 min CN=46 Runoff=0.10 cfs 0.023 af
Subcatchment 5: WS-5	Runoff Area=31,606 sf 25.00% Impervious Runoff Depth=0.65" Flow Length=385' Tc=9.6 min CN=54 Runoff=0.31 cfs 0.039 af
Subcatchment 6: WS-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=0.65" Flow Length=698' Tc=13.0 min CN=54 Runoff=1.28 cfs 0.175 af
Reach S1: north boundary swale	Avg. Flow Depth=0.06' Max Vel=1.85 fps Inflow=0.45 cfs 0.045 af n=0.030 L=256.0' S=0.0684 '/' Capacity=69.96 cfs Outflow=0.43 cfs 0.045 af
Reach S2: south boundary swal	e Avg. Flow Depth=0.15' Max Vel=3.11 fps Inflow=2.05 cfs 0.188 af n=0.030 L=266.0' S=0.0583 '/' Capacity=64.60 cfs Outflow=2.03 cfs 0.188 af
Pond C1: CULVERT 1	Peak Elev=276.00' Storage=0 cf Inflow=0.10 cfs 0.023 af 18.0" Round Culvert n=0.010 L=33.0' S=0.0379 '/' Outflow=0.10 cfs 0.023 af
Pond C2: CULVERT 2	Peak Elev=266.19' Inflow=2.03 cfs 0.188 af 18.0" Round Culvert n=0.010 L=8.0' S=0.7813 '/' Outflow=2.03 cfs 0.188 af
Pond CB-1A: CB-1	Peak Elev=276.42' Inflow=1.57 cfs 0.214 af 12.0" Round Culvert n=0.010 L=122.0' S=0.0100 '/' Outflow=1.57 cfs 0.214 af

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Pond CB-2: CB-2	Peak Elev=271.89' Inflow=2.13 cfs 0.322 af 18.0" Round Culvert n=0.010 L=175.0' S=0.0310 '/' Outflow=2.13 cfs 0.322 af
Pond CB-3A: CB-3A	Peak Elev=272.78' Inflow=0.57 cfs 0.108 af 18.0" Round Culvert n=0.010 L=51.0' S=0.0284 '/' Outflow=0.57 cfs 0.108 af
Pond CB-4A: CB-1	Peak Elev=274.06' Inflow=0.39 cfs 0.054 af 12.0" Round Culvert n=0.010 L=119.5' S=0.0100 '/' Outflow=0.39 cfs 0.054 af
Pond CB-5: CB-5	Peak Elev=266.44' Inflow=2.53 cfs 0.367 af 18.0" Round Culvert n=0.010 L=18.0' S=0.0222 '/' Outflow=2.53 cfs 0.367 af
Pond CB-6: CB-6	Peak Elev=266.46' Inflow=2.03 cfs 0.189 af 18.0" Round Culvert n=0.010 L=49.0' S=0.0100 '/' Outflow=2.03 cfs 0.189 af
Pond CB-7: CB-7	Peak Elev=266.42' Storage=83 cf Inflow=2.03 cfs 0.190 af 18.0" Round Culvert n=0.010 L=82.0' S=0.0100 '/' Outflow=2.03 cfs 0.191 af
Pond CB-8: CB-8	Peak Elev=266.38' Inflow=2.03 cfs 0.191 af 18.0" Round Culvert n=0.010 L=42.0' S=0.0057 '/' Outflow=2.03 cfs 0.191 af
Pond DMH-1A: DMH-1A	Peak Elev=274.79' Inflow=1.57 cfs 0.214 af 12.0" Round Culvert n=0.010 L=17.5' S=0.0097 '/' Outflow=1.57 cfs 0.214 af
Pond DMH-3A: DMH-3A	Peak Elev=271.57' Inflow=2.13 cfs 0.322 af 18.0" Round Culvert n=0.010 L=82.0' S=0.2091 '/' Outflow=2.13 cfs 0.322 af
Pond DMH-4: DMH-4	Peak Elev=266.35' Inflow=6.12 cfs 0.740 af 10.0" Round Culvert n=0.013 L=200.0' S=0.0300 '/' Outflow=6.12 cfs 0.740 af
Total Runoff	Area = 9.490 ac Runoff Volume = 0.739 af Average Runoff Depth = 0.93" 67.35% Pervious = 6.391 ac 32.65% Impervious = 3.099 ac

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#### Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1A: WS-1A	Runoff Area=45,731 sf 54.20% Impervious Runoff Depth=2.12" Flow Length=508' Tc=9.6 min CN=71 Runoff=2.24 cfs 0.185 af
Subcatchment 1B: WS-1B	Runoff Area=43,860 sf 24.41% Impervious Runoff Depth=0.92" Flow Length=607' Tc=14.1 min CN=54 Runoff=0.64 cfs 0.077 af
Subcatchment 1C: WS-1C	Runoff Area=27,533 sf 32.94% Impervious Runoff Depth=1.17" Flow Length=351' Tc=9.0 min CN=58 Runoff=0.66 cfs 0.062 af
Subcatchment 2A: WS-2A	Runoff Area=70,105 sf 48.25% Impervious Runoff Depth=1.80" Flow Length=466' Slope=0.0460 '/' Tc=11.5 min CN=67 Runoff=2.71 cfs 0.242 af
Subcatchment 2B: WS-2B	Runoff Area=2,611 sf 11.18% Impervious Runoff Depth=0.49" Flow Length=132' Slope=0.0250 '/' Tc=9.9 min CN=46 Runoff=0.01 cfs 0.002 af
Subcatchment 2C: WS-2C	Runoff Area=4,493 sf 100.00% Impervious Runoff Depth=4.76" Flow Length=267' Tc=1.7 min CN=98 Runoff=0.55 cfs 0.041 af
Subcatchment 2D: WS-2D	Runoff Area=2,894 sf 0.00% Impervious Runoff Depth=0.20" Flow Length=64' Slope=0.2000 '/' Tc=2.9 min CN=39 Runoff=0.00 cfs 0.001 af
Subcatchment 3A: WS-3A	Runoff Area=3,800 sf 100.00% Impervious Runoff Depth=4.76" Flow Length=181' Tc=1.7 min CN=98 Runoff=0.46 cfs 0.035 af
Subcatchment 3B: WS-3B	Runoff Area=39,994 sf 12.31% Impervious Runoff Depth=0.49" Flow Length=585' Tc=14.0 min CN=46 Runoff=0.20 cfs 0.037 af
Subcatchment 5: WS-5	Runoff Area=31,606 sf 25.00% Impervious Runoff Depth=0.92" Flow Length=385' Tc=9.6 min CN=54 Runoff=0.53 cfs 0.056 af
Subcatchment 6: WS-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=0.92" Flow Length=698' Tc=13.0 min CN=54 Runoff=2.11 cfs 0.248 af
Reach S1: north boundary swa	ale         Avg. Flow Depth=0.07'         Max Vel=2.14 fps         Inflow=0.66 cfs         0.062 af           n=0.030         L=256.0'         S=0.0684 '/'         Capacity=69.96 cfs         Outflow=0.65 cfs         0.062 af
Reach S2: south boundary sw	ale Avg. Flow Depth=0.17' Max Vel=3.42 fps Inflow=2.71 cfs 0.242 af n=0.030 L=266.0' S=0.0583 '/' Capacity=64.60 cfs Outflow=2.68 cfs 0.242 af
Pond C1: CULVERT 1	Peak Elev=276.59' Storage=56 cf Inflow=0.20 cfs 0.037 af 18.0" Round Culvert n=0.010 L=33.0' S=0.0379 '/' Outflow=0.98 cfs 0.039 af
Pond C2: CULVERT 2	Peak Elev=296.61' Inflow=2.68 cfs 0.242 af 18.0" Round Culvert n=0.010 L=8.0' S=0.7813 '/' Outflow=2.68 cfs 0.242 af
Pond CB-1A: CB-1	Peak Elev=297.50' Inflow=2.60 cfs 0.303 af 12.0" Round Culvert n=0.010 L=122.0' S=0.0100 '/' Outflow=2.60 cfs 0.303 af

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Pond CB-2: CB-2	Peak Elev=296.90' Inflow=3.54 cfs 0.454 af 18.0" Round Culvert n=0.010 L=175.0' S=0.0310 '/' Outflow=3.54 cfs 0.454 af	
Pond CB-3A: CB-3A	Peak Elev=296.90' Inflow=1.49 cfs 0.151 af 18.0" Round Culvert n=0.010 L=51.0' S=0.0284 '/' Outflow=1.49 cfs 0.151 af	
Pond CB-4A: CB-1	Peak Elev=296.93' Inflow=0.64 cfs 0.077 af 12.0" Round Culvert n=0.010 L=119.5' S=0.0100 '/' Outflow=0.64 cfs 0.077 af	
Pond CB-5: CB-5	Peak Elev=296.59' Inflow=4.15 cfs 0.516 af 18.0" Round Culvert n=0.010 L=18.0' S=0.0222 '/' Outflow=4.15 cfs 0.516 af	
Pond CB-6: CB-6	Peak Elev=296.57' Inflow=2.69 cfs 0.244 af 18.0" Round Culvert n=0.010 L=49.0' S=0.0100 '/' Outflow=2.66 cfs 0.244 af	
Pond CB-7: CB-7	Peak Elev=296.52' Storage=83 cf Inflow=2.66 cfs 0.245 af 18.0" Round Culvert n=0.010 L=82.0' S=0.0100 '/' Outflow=3.57 cfs 0.249 af	
Pond CB-8: CB-8	Peak Elev=296.40' Inflow=3.57 cfs 0.249 af 18.0" Round Culvert n=0.010 L=42.0' S=0.0057 '/' Outflow=3.57 cfs 0.249 af	
Pond DMH-1A: DMH-1A	Peak Elev=297.18' Inflow=2.60 cfs 0.303 af 12.0" Round Culvert n=0.010 L=17.5' S=0.0097 '/' Outflow=2.60 cfs 0.303 af	
Pond DMH-3A: DMH-3A	Peak Elev=296.74' Inflow=3.54 cfs 0.454 af 18.0" Round Culvert n=0.010 L=82.0' S=0.2091 '/' Outflow=3.54 cfs 0.454 af	
Pond DMH-4: DMH-4	Peak Elev=296.37' Inflow=9.89 cfs 0.991 af 10.0" Round Culvert n=0.013 L=200.0' S=0.0300 '/' Outflow=9.89 cfs 0.991 af	
Total Runoff Area = 9.490 ac Runoff Volume = 0.985 af Average Runoff Depth = 1.25" 67.35% Pervious = 6.391 ac 32.65% Impervious = 3.099 ac		

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#### Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment 1A: WS-1A	Runoff Area=45,731 sf 54.20% Impervious Runoff Depth=2.50" Flow Length=508' Tc=9.6 min CN=71 Runoff=2.66 cfs 0.219 af
Subcatchment 1B: WS-1B	Runoff Area=43,860 sf 24.41% Impervious Runoff Depth=1.17" Flow Length=607' Tc=14.1 min CN=54 Runoff=0.87 cfs 0.098 af
Subcatchment 1C: WS-1C	Runoff Area=27,533 sf 32.94% Impervious Runoff Depth=1.45" Flow Length=351' Tc=9.0 min CN=58 Runoff=0.86 cfs 0.077 af
Subcatchment 2A: WS-2A	Runoff Area=70,105 sf 48.25% Impervious Runoff Depth=2.16" Flow Length=466' Slope=0.0460 '/' Tc=11.5 min CN=67 Runoff=3.29 cfs 0.290 af
Subcatchment 2B: WS-2B	Runoff Area=2,611 sf 11.18% Impervious Runoff Depth=0.67" Flow Length=132' Slope=0.0250 '/' Tc=9.9 min CN=46 Runoff=0.02 cfs 0.003 af
Subcatchment 2C: WS-2C	Runoff Area=4,493 sf 100.00% Impervious Runoff Depth=5.26" Flow Length=267' Tc=1.7 min CN=98 Runoff=0.60 cfs 0.045 af
Subcatchment 2D: WS-2D	Runoff Area=2,894 sf 0.00% Impervious Runoff Depth=0.31" Flow Length=64' Slope=0.2000 '/' Tc=2.9 min CN=39 Runoff=0.01 cfs 0.002 af
Subcatchment 3A: WS-3A	Runoff Area=3,800 sf 100.00% Impervious Runoff Depth=5.26" Flow Length=181' Tc=1.7 min CN=98 Runoff=0.51 cfs 0.038 af
Subcatchment 3B: WS-3B	Runoff Area=39,994 sf 12.31% Impervious Runoff Depth=0.67" Flow Length=585' Tc=14.0 min CN=46 Runoff=0.31 cfs 0.051 af
Subcatchment 5: WS-5	Runoff Area=31,606 sf 25.00% Impervious Runoff Depth=1.17" Flow Length=385' Tc=9.6 min CN=54 Runoff=0.72 cfs 0.071 af
Subcatchment 6: WS-6	Runoff Area=140,742 sf 25.00% Impervious Runoff Depth=1.17" Flow Length=698' Tc=13.0 min CN=54 Runoff=2.89 cfs 0.315 af
Reach S1: north boundary swa	ale         Avg. Flow Depth=0.08'         Max Vel=2.37 fps         Inflow=0.86 cfs         0.077 af           n=0.030         L=256.0'         S=0.0684 '/'         Capacity=69.96 cfs         Outflow=0.85 cfs         0.077 af
Reach S2: south boundary sw	ale         Avg. Flow Depth=0.19'         Max Vel=3.66 fps         Inflow=3.29 cfs         0.290 af           n=0.030         L=266.0'         S=0.0583 '/'         Capacity=64.60 cfs         Outflow=3.25 cfs         0.290 af
Pond C1: CULVERT 1	Peak Elev=277.39' Storage=265 cf Inflow=0.31 cfs 0.051 af 18.0" Round Culvert n=0.010 L=33.0' S=0.0379 '/' Outflow=2.90 cfs 0.056 af
Pond C2: CULVERT 2	Peak Elev=324.46' Inflow=3.25 cfs 0.290 af 18.0" Round Culvert n=0.010 L=8.0' S=0.7813 '/' Outflow=3.25 cfs 0.290 af
Pond CB-1A: CB-1	Peak Elev=326.05' Inflow=3.56 cfs 0.386 af 12.0" Round Culvert n=0.010 L=122.0' S=0.0100 '/' Outflow=3.56 cfs 0.386 af

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Pond CB-2: CB-2	Peak Elev=325.03' Inflow=4.93 cfs 0.578 af 18.0" Round Culvert n=0.010 L=175.0' S=0.0310 '/' Outflow=4.93 cfs 0.578 af	
Pond CB-3A: CB-3A	Peak Elev=325.04' Inflow=3.38 cfs 0.193 af 18.0" Round Culvert n=0.010 L=51.0' S=0.0284 '/' Outflow=3.38 cfs 0.193 af	
Pond CB-4A: CB-1	Peak Elev=325.08' Inflow=0.87 cfs 0.098 af 12.0" Round Culvert n=0.010 L=119.5' S=0.0100 '/' Outflow=0.87 cfs 0.098 af	
Pond CB-5: CB-5	Peak Elev=324.53' Inflow=5.68 cfs 0.655 af 18.0" Round Culvert n=0.010 L=18.0' S=0.0222 '/' Outflow=5.68 cfs 0.655 af	
Pond CB-6: CB-6	Peak Elev=324.41' Inflow=3.27 cfs 0.293 af 18.0" Round Culvert n=0.010 L=49.0' S=0.0100 '/' Outflow=3.24 cfs 0.293 af	
Pond CB-7: CB-7	Peak Elev=324.34' Storage=83 cf Inflow=3.25 cfs 0.295 af 18.0" Round Culvert n=0.010 L=82.0' S=0.0100 '/' Outflow=4.15 cfs 0.298 af	
Pond CB-8: CB-8	Peak Elev=324.18' Inflow=4.15 cfs 0.298 af 18.0" Round Culvert n=0.010 L=42.0' S=0.0057 '/' Outflow=4.15 cfs 0.298 af	
Pond DMH-1A: DMH-1A	Peak Elev=325.52' Inflow=3.56 cfs 0.386 af 12.0" Round Culvert n=0.010 L=17.5' S=0.0097 '/' Outflow=3.56 cfs 0.386 af	
Pond DMH-3A: DMH-3A	Peak Elev=324.80' Inflow=4.93 cfs 0.578 af 18.0" Round Culvert n=0.010 L=82.0' S=0.2091 '/' Outflow=4.93 cfs 0.578 af	
Pond DMH-4: DMH-4	Peak Elev=324.12' Inflow=12.40 cfs 1.217 af 10.0" Round Culvert n=0.013 L=200.0' S=0.0300 '/' Outflow=12.40 cfs 1.217 af	
Total Runoff Area = 9.490 ac Runoff Volume = 1.209 af Average Runoff Depth = 1.53" 67.35% Pervious = 6.391 ac 32.65% Impervious = 3.099 ac		

APPENDIX D

